MEMORANDUM

TO: Mr. Joseph Lynch **FROM:** Mr. Jeffrey S. Dirk, P.E., PTOE, FITE and

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DATE: August 7, 2014 **RE:** 6810

SUBJECT: Transportation Impact Assessment

Proposed Wyman Village Residential Community

Community Way

Foxborough, Massachusetts

Vanasse & Associates, Inc. (VAI) has conducted a Transportation Impact Assessment (TIA) in order to determine the potential impacts on the transportation infrastructure associated with the proposed construction of the Wyman Village residential community to be located off Community Way in Foxborough, Massachusetts (hereafter referred to as the "Project"). This study evaluates the following specific areas as they relate to the Project: i) access requirements; ii) potential off-site improvements; and iii) safety considerations; and identifies and analyzes existing traffic conditions and future traffic conditions, both with and without the Project, along Cocasset Street and Community Way, as well as at the intersection of the two roadways.

This study was prepared in consultation with the Massachusetts Department of Transportation (MassDOT) and the Town of Foxborough; was performed in general accordance with MassDOT's *Transportation Impact Assessment (TIA) Guidelines*; and was conducted pursuant to the standards of the Traffic Engineering and Transportation Planning professions for the preparation of such reports.

PROJECT DESCRIPTION

The Project will entail the construction of a residential community consisting of 20 single-family homes to be located off Community Way in Foxborough, Massachusetts. The Project site consists of approximately four-acres of land and is bounded by residential properties and areas of open and wooded space to the north and west; residential properties and Community Way to the south; and residential properties, areas of open and wooded space and a Massachusetts Bay Transportation Authority (MBTA) railroad right-of-way to the east. Figure 1 depicts the Project site location in relation to the existing roadway network.

Access to the Project site will be provided by a new roadway that will intersect the north side of Community Way and will be aligned opposite the eastern connection between Community Way and Cocasset Street.





Vanasse & Associates, Inc.
Transportation Engineers & Planners

EXISTING CONDITIONS

A comprehensive field inventory of existing conditions within the study area was conducted in July 2014. The field investigation consisted of an inventory of existing roadway geometrics; pedestrian and bicycle facilities; public transportation services; traffic volumes; and operating characteristics; as well as posted speed limits and land use information within the study area. The study area for the Project was selected to contain the roadways providing access to the Project site (Cocasset Street and Community Way), as well as the intersection of these two roadways. It should be noted that Cocasset Street was recently paved at the time that the field inventories that form the basis of this assessment were completed and pavement markings were not yet installed.

The following describes the study area roadways and intersection.

Roadway

Cocasset Street

- Two-lane, urban minor arterial roadway under local jurisdiction
- > Traverses study area in a general northwest-southeast direction
- ➤ Provides two 12 to 13-foot wide travel lanes separated by a double-yellow centerline
- ➤ Provides 2-foot wide marked shoulders along both sides
- A sidewalk is provided along the north side within the study area
- > Illumination is provided by way of street lights mounted on wood poles
- ➤ Posted speed limit is 35 miles per hour (mph) within the study area (20 mph within the school zone located just west of the Cocasset Street/Community Way intersection during school hours)
- Land use consists of residential properties and areas of open and wooded space

Community Way

- > Two-lane, dead-end roadway under local jurisdiction
- > Traverses study area in a general east-west direction
- > Consists of a 22-foot wide paved roadway with no marked centerline or shoulders provided
- ➤ A sidewalk is not provided along the roadway
- > Illumination is provided by way of street lights mounted on wood poles near Cocasset Street
- Land use consists of the Project site, residential properties and areas of open and wooded space

Intersection

Cocasset Street at Community Way

- Community Way forms two (2) separate three-legged, unsignalized intersections with the Community Way under assumed stop control (STOP-signs are not currently provided)
- ➤ The Cocasset Street northwest and southeastbound approaches provide one 12 to 13-foot wide general-purpose travel lane with 2-foot wide marked shoulders



- > The directions of travel along Cocasset Street are separated by a double-yellow centerline
- ➤ Community Way is a 22-foot wide roadway that accommodates two-way travel with no marked centerline or shoulders provided that intersects Cocasset Street at two (2) separate locations separated by a large triangular shaped island. Both Community Way approaches are under assumer stop control; however, STOP-signs are not currently provided.
- > Sidewalks are provided along the north side of Cocasset Street with crosswalks provided across Community Way
- ➤ A "School Zone Ahead" sign is located on the Cocasset Street northwestbound approach in the vicinity of the intersection
- ➤ Land use in the vicinity of the intersection consists of the Project site, residential properties, and areas of open and wooded space

Existing Traffic Volumes

In order to determine existing traffic-volume demands and flow patterns within the study area, automatic traffic recorder (ATR) counts, manual turning movement counts (TMCs) and vehicle classification counts were completed in July 2014. The ATR counts were conducted on Cocasset Street in the vicinity of the Project site in order to record weekday daily traffic conditions over an extended period, with weekday morning (7:00 to 9:00 AM) and evening (4:00 to 6:00 PM) peak period manual TMCs performed at the study intersection. These time periods were selected for analysis purposes as they are representative of the peak traffic volume hours for both the Project and the adjacent roadway network.

Seasonal Adjustment

In order to evaluate the potential for seasonal fluctuation of traffic volumes within the study area, MassDOT weekday seasonal factors for Group 6 roadways (urban arterials, collectors and rural arterials, the MassDOT functional classification for Main Street) were reviewed. Based on a review of this data, it was determined that traffic volumes for the month of July are approximately 8.7 percent above average-month conditions; however, public schools were not in session at the time that the traffic counts were performed (July). Given the proximity of the Mabelle M. Burrell Elementary School to the Project site (located off Cocasset Street approximately ¼-mile west of the Project site), traffic volumes associated with the school were estimated using trip-generation statistics published by the Institute of Transportation Engineers (ITE)² and assigned onto the study area roadway network based on existing traffic patterns in order to provide an appropriate analysis condition reflective of conditions when public schools are in session. The 2014 Existing weekday morning and evening peak-hour traffic volumes are summarized in Table 1 and graphically depicted on Figure 2.

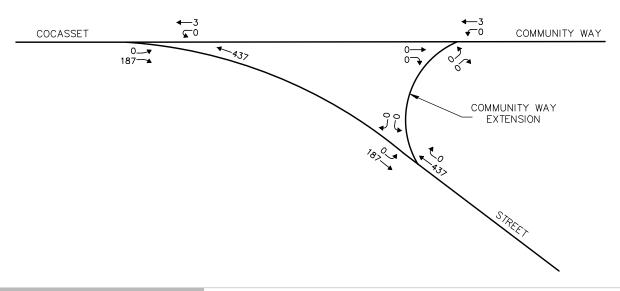
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¹MassDOT Traffic Volumes for the Commonwealth of Massachusetts; 2011 Weekday Seasonal Factors, Group 6 – Urban Arterials, Collectors and Rural Arterials.

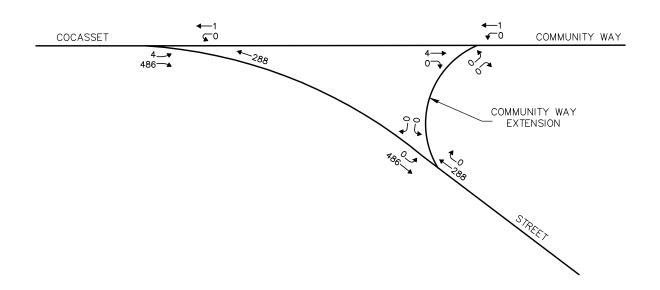
²Trip Generation, 9th Edition; Institute of Transportation Engineers; Washington, DC; 2012.





WEEKDAY EVENING PEAK HOUR







2014 Existing
Peak Hour Traffic Volumes

Figure 2

Table 1 2014 EXISTING TRAFFIC VOLUMES

		Week	tday Morning 1 (7:30 – 8:30 A			day Evening 1 (4:45 – 5:45	
Location	AWT ^a	VPH ^b	K Factor ^c	Directional Distribution	VPH	K Factor	Directional Distribution
Cocasset Street, east of Community Way	8,515	624	7.3	70.0% WB	774	9.1	62.8% EB

^aAverage weekday traffic in vehicles per day.

EB = eastbound; WB = westbound.

As can be seen in Table 1, Cocasset Street in the vicinity of the Project site was found to accommodate approximately 8,515 vehicles on an average weekday (two-way, 24-hour volume), with approximately 624 vehicles per hour (vph) during the weekday morning peak-hour (7:30 to 8:30 AM) and 774 vph during the weekday evening peak-hour (4:45 to 5:45 PM).

Pedestrian and Bicycle Facilities

A comprehensive field inventory of pedestrian and bicycle facilities within the study area was undertaken in July 2014. The field inventory consisted of a review of the location of sidewalks and pedestrian crossing locations along the study roadways and at the study intersection, as well as the location of existing and planned future bicycle facilities. Sidewalks are provided along the north side of Cocasset Street within the study area, with crosswalks provided across Community Way.

Formal bicycle facilities were not identified within the study area; however, Cocasset Street provides sufficient width (combined travel lane and shoulder) to support bicycle travel in a shared travelled-way configuration.³

Public Transportation

Public transportation services are provided to the Town of Foxborough by the Massachusetts Bay Transportation Authority (MBTA) and the Greater Attleborough-Taunton Regional Transit Authority (GATRA); however, these services are currently not available within the immediate study area.

Spot Speed Measurements

Vehicle travel speed measurements were performed on Cocasset Street in the vicinity of the Project site over a continuous 48-hour period in conjunction with the ATR counts. Table 2 summarizes the vehicle travel speed measurements.

^bVehicles per hour.

^cPercent of daily traffic occurring during the peak-hour.

³A minimum combined travel lane and paved shoulder width of 14-feet is required to support bicycle travel in a shared travelled-way condition.

Table 2 VEHICLE TRAVEL SPEED MEASUREMENTS

	Cocass	et Street
	Eastbound	Westbound
Mean Travel Speed (mph)	38	38
85 th Percentile Speed (mph)	41	42
Posted Speed Limit (mph)	35	35

mph = miles per hour.

As can be seen in Table 2, the mean (average) vehicle travel speed along Cocasset Street in the vicinity of the Project site was found to be approximately 38 mph. The average measured 85th percentile vehicle travel speed, or the speed at which 85 percent of the observed vehicles traveled at or below, was found to be approximately 41 mph, which is approximately 6 mph above the posted speed limit (35 mph). The 85th percentile speed is used as the basis of engineering design and in the evaluation of sight distances, and is often used in establishing posted speed limits.

Motor Vehicle Crash Data

Motor vehicle crash information for the study area intersection was provided by the MassDOT Highway Division Safety Management/Traffic Operations Unit for the most recent five-year period available (2008 through 2012, inclusive) in order to examine motor vehicle crash trends occurring within the study area. The data is summarized by intersection, type, severity, and day of occurrence, and presented in Table 3.



Table 3
MOTOR VEHICLE CRASH DATA SUMMARY^a

	Cocasset Street/ Community Way
Traffic Control Type ^b	U
Year:	
2008	1
2009	0
2010	0
2011	0
<u>2012</u>	<u>0</u>
Total	1
Average	0.20
Calculated Rate ^c	0.06
MassDOT Crash Rate ^d	0.60/0.58
Significant? ^e	No
Туре:	
Angle	0
Rear-End	1
Head-On	0
Sideswipe	0
Fixed Object	0
Pedestrian/Bicycle	0
<u>Unknown/Other</u>	<u>0</u> 1
Total	1
Day of Week:	
Monday through Friday	1
Saturday	0
Sunday	<u>0</u> 1
Total	1
Severity:	
Property Damage Only	1
Personal Injury	0
<u>Fatality</u>	<u>0</u>
Total	1

^aSource: MassDOT Safety Management/Traffic Operations Unit records, 2008 through 2012.

As can be seen in Table 3, one (1) motor vehicle crash was reported at the Cocasset Street/Community Way intersection over the five-year review period. The crash was reported as a rear-end-type collision and resulted in property damage only. The Cocasset Street/Community Way intersection was found to have a motor vehicle crash rate <u>below</u> both the MassDOT statewide and Highway Division District 5 (the Highway Division District in which the intersection is located) average crash rate for an unsignalized



^bTraffic Control Type: U = unsignalized.

^cCrash rate per million vehicles entering the intersection.

^dStatewide/District crash rate.

^eThe intersection crash rate is significant if it is found to exceed MassDOT statewide or District (District 5) crash rate.

intersection. No fatal motor vehicle crashes were reported within the study area over the five-year review period. The detailed MassDOT Crash Rate Worksheet is provided in the Appendix.

Based on a review of the MassDOT motor vehicle crash data, no discernible safety deficiencies were apparent at the study intersection.

FUTURE CONDITIONS

Traffic volumes in the study area were projected to the year 2021, which reflects a seven-year planning horizon consistent with MassDOT's *Transportation Impact Assessment (TIA) Guidelines*. Independent of the Project, traffic volumes on the roadway network in the year 2021 under No-Build conditions include all existing traffic and new traffic resulting from background traffic growth. Anticipated Project-generated traffic volumes superimposed upon the 2021 No-Build traffic volumes reflect 2021 Build traffic volume conditions with the Project.

Background Traffic Growth

Future traffic growth is a function of the expected land development in the immediate area and the surrounding region. Several methods can be used to estimate this growth. A procedure frequently employed estimates an annual percentage increase in traffic growth and applies that percentage to all traffic volumes under study. The drawback to such a procedure is that some turning volumes may actually grow at either a higher or a lower rate at particular intersections.

An alternative procedure identifies the location and type of planned development, estimates the traffic to be generated, and assigns it to the area roadway network. This procedure produces a more realistic estimate of growth for local traffic. However, the drawback of this procedure is that the potential growth in population and development external to the study area would not be accounted for in the traffic projections.

To provide a conservative analysis framework, both procedures were used, the salient components of which are described below.

Specific Development by Others

The Planning Departments of the Towns of Foxborough, Mansfield and Sharon were contacted in order to determine if there were any projects planned within the study area that would have an impact on future traffic volumes at the study intersection. Based on these discussions, the following projects were identified for inclusion in this assessment:

• River Ridge Residential Development, Cocasset Street, Foxborough, Massachusetts. This project will entail the construction of a 40-lot residential community to be located off Cocasset Street and west of the Project site in Foxborough, Massachusetts. Traffic volumes expected to be generated by this project were estimated using trip-generation statistics published by the ITE⁴ for a similar land use as that proposed and assigned onto the study area roadway network based on existing traffic patterns.



⁴Ibid 2.

- Redevelopment of the "Chocolate Factory", 150 Oak Street, Mansfield, Massachusetts. This project will entail the redevelopment of the "Chocolate Factory" located at 150 Oak Street in Mansfield, Massachusetts, to include approximately 130 residential apartment units. Traffic volumes associated with this project are not expected to result in an increase in traffic within the study area that would exceed the background traffic growth rate (discussion follows).
- Sharon Commons Mixed-Use Development, South Main Street, Sharon, Massachusetts. This project will entail the construction of approximately 192 residential apartment units and 450,000 square feet (sf) of retail space to be located off South Main Street in Sharon, Massachusetts. Traffic volumes associated with this project are not expected to result in an increase in traffic within the study area that would exceed the background traffic growth rate.

No other developments were identified at this time that are expected to result in an increase in traffic within the study area beyond the general background traffic growth rate.

General Background Traffic Growth

Traffic-volume data compiled by MassDOT from permanent count stations and historic traffic counts in the area were reviewed in order to determine general background traffic growth trends. Based on a review of this data, it was determined that traffic volumes within the study area have generally decreased by approximately 1.4 percent over the past several years. In order to provide a conservative (high) analysis scenario and a prudent planning condition for the Project, a 1.0 percent per year compounded annual background traffic growth rate was used in order to account for future traffic growth and presently unforeseen development within the study area.

Roadway Improvement Projects

MassDOT and the Town of Foxborough were contacted in order to determine if there were any planned roadway improvement projects expected to be completed within the study area. Based on these discussions, no roadway improvement projects outside of routine maintenance activities were identified to be planned within the study area at this time that are expected to result in a material impact on traffic volumes or operating conditions within the study area that is the subject of this assessment. As mentioned previously, Cocasset Street was repaved in July 2014 and pavement markings were in the process of being installed at the time that the field inventory that forms the basis of this assessment was completed.

No-Build Traffic Volumes

The 2021 No-Build condition peak-hour traffic-volumes were developed by applying the 1.0 percent per year compounded annual background traffic growth rate to the 2014 Existing peak-hour traffic volumes and then adding the peak hour traffic volumes associated with the identified specific development project by others. The resulting 2021 No-Build weekday morning and evening peak-hour traffic volumes are shown on Figure 3.

Project-Generated Traffic

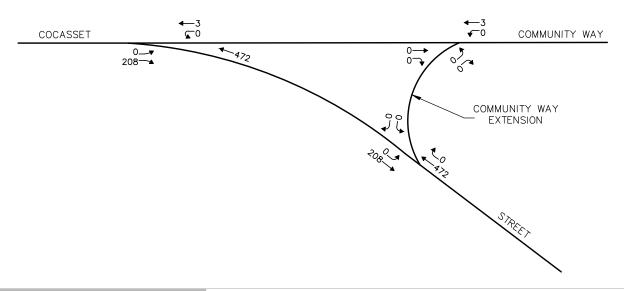
As proposed, the Project will entail the construction of a residential community consisting of 20 single-family homes. In order to develop the traffic characteristics of the Project, trip-generation statistics published by the ITE⁵ for a similar land use as that proposed were used. ITE Land Use Code



⁵Ibid 2.

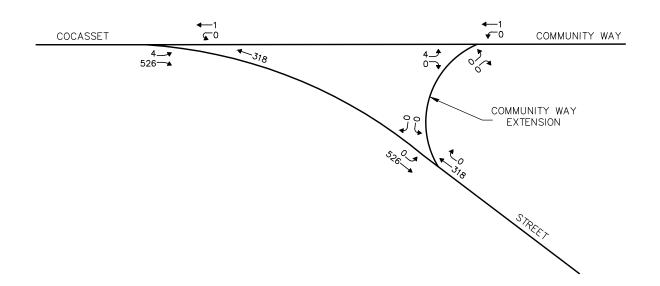
WEEKDAY MORNING PEAK HOUR





WEEKDAY EVENING PEAK HOUR







2021 No-Build Peak Hour Traffic Volumes

Figure 3

(LUC) 210, *Single-Family Detached Housing*, with the independent variable of number of dwelling units equal to 20, was used to develop the traffic characteristics of the Project. Table 4 summarizes the anticipated traffic characteristics of the Project.

Table 4
TRIP GENERATION SUMMARY

	Vehicle Trips
Time Period/Direction	Wyman Village Residential Community (20 units) ^a
Average Weekday Daily:	
Entering	120
Exiting	<u>120</u>
Total	240
Weekday Morning Peak Hour:	
Entering	6
Exiting	<u>18</u>
Total	$\overline{24}$
Weekday Evening Peak Hour:	
Entering	16
Exiting	<u>9</u>
Total	$\overline{25}$

^aBased on ITE LUC 210, Single-Family Detached Housing.

As can be seen in Table 4, the Project is expected to generate approximately 240 vehicle trips (two-way, 24-hour volume, or 120 vehicles entering and 120 exiting), with approximately 24 vehicle trips (6 vehicles entering and 18 exiting) expected during the weekday morning peak-hour and 25 vehicle trips (16 vehicles entering and 9 exiting) expected during the weekday evening peak-hour.

Such traffic volume increases (approximately one (1) additional vehicle every 2.5 minutes during the peak hours) would not result in a significant impact (increase) on motorist delays or vehicle queuing over existing or anticipated future conditions without the Project.

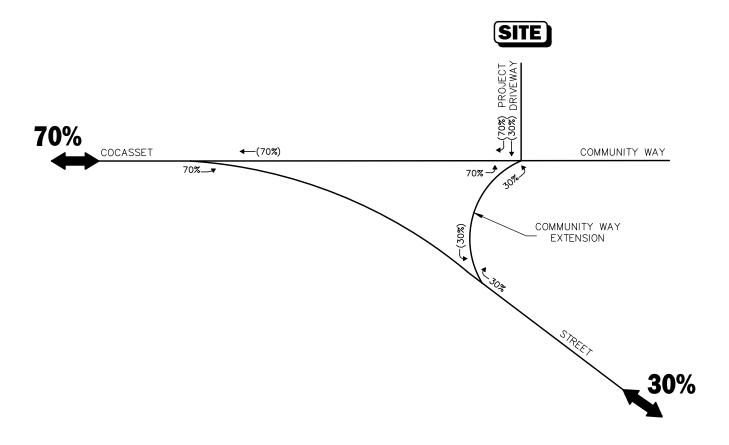
Trip Distribution and Assignment

The directional distribution of generated trips to and from the Project site was determined based on a review of existing traffic patterns within the study area during the commuter peak periods. The general trip distribution for the Project is graphically depicted on Figure 4. The additional traffic expected to be generated by the Project was assigned on the study area roadway network as shown on Figure 5.

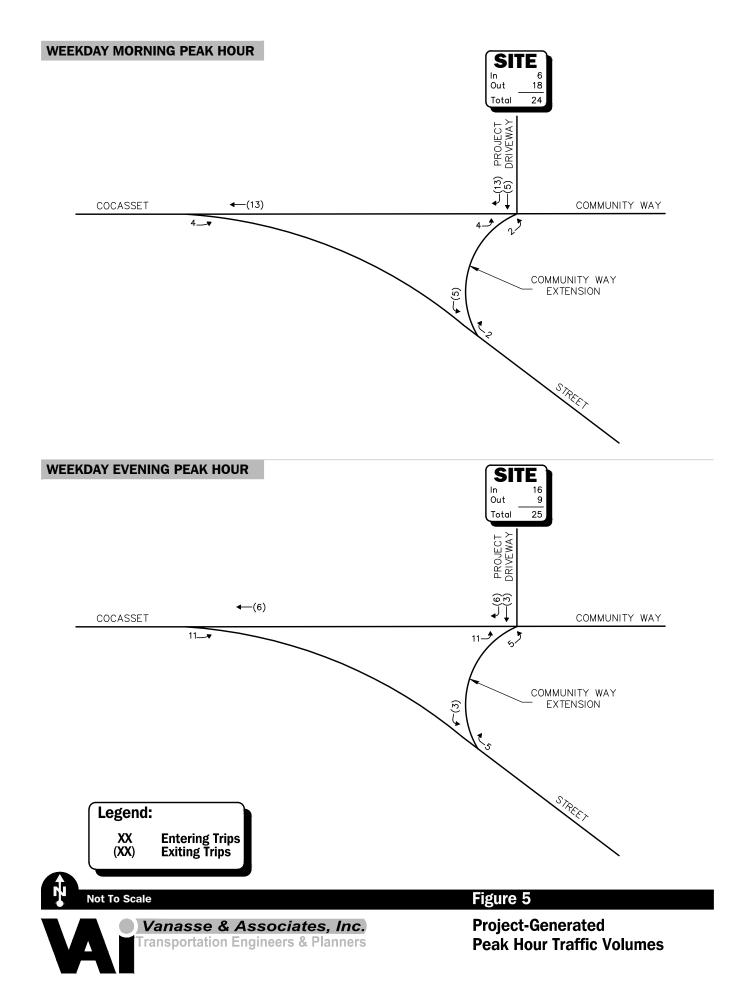


Legend:

XX Entering Trips (XX) Exiting Trips







Build Traffic Volumes

The 2021 Build condition traffic volumes consist of the 2021 No-Build traffic volumes with the additional traffic expected to be generated by the Project added to them. The 2021 Build weekday morning and evening peak-hour traffic-volumes are graphically depicted on Figure 6.

A summary of peak-hour projected traffic-volume increases external to the study area that is the subject of this assessment is shown in Table 5. These volumes are based on the expected increases from the Project.

Table 5
PEAK-HOUR TRAFFIC-VOLUME INCREASES

Location/Peak Hour	2014 Existing	2021 No-Build	2021 Build	Traffic Volume Increase Over No-Build	Percent Increase Over No-Build
Cocasset Street, east of Community Way					
Weekday Morning	624	680	687	7	1.0
Weekday Evening	774	844	852	8	0.9
•					
Cocasset Street, west of Community Way					
Cocasset Street, west of Community Way Weekday Morning	627	683	700	17	2.5

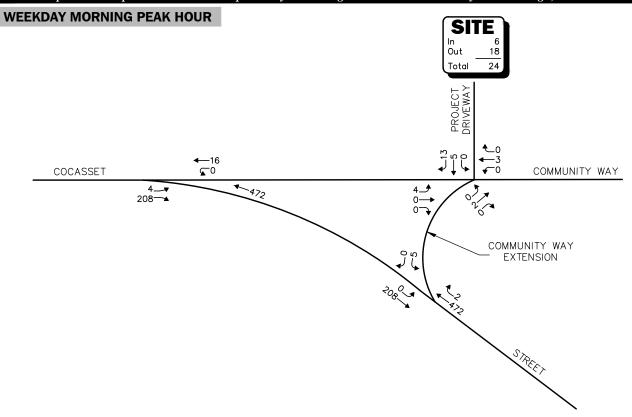
As shown in Table 5, Project-related traffic-volume increases external to the study area relative to 2021 No-Build conditions are anticipated to range from 0.9 to 2.5 percent during the peak periods, with vehicle increases shown to range from 7 to 17 vehicles. Such increases are considered nominal when distributed over the course of an hour and would not be readily apparent outside of the immediate study area that is the subject of this assessment.

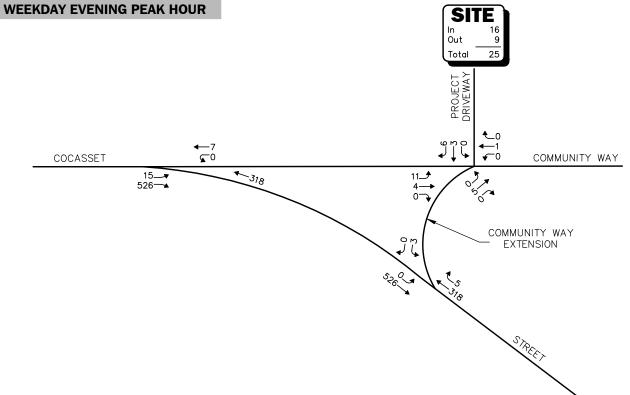
TRAFFIC OPERATIONS ANALYSIS

In order to assess the potential impact of the Project on the roadway network, traffic operations analyses (level-of-service, motorist delay and vehicle queuing) were performed for the study intersections under 2014 Existing, 2021 No-Build, and 2021 Build conditions.

In brief, six levels of service are defined for each type of facility and are correlated to motorist delay. They are given letter designations ranging from A to F, with level-of-service (LOS) A representing the best operating conditions and LOS F representing congested or constrained operations. Since the level-of-service of a traffic facility is a function of the flows placed upon it, such a facility may operate at a wide range of levels of service depending on the time of day, day of week, or period of the year. The *Synchro* intersection capacity analysis software, which is based on the analysis methodologies and









Peak Hour Traffic Volumes

procedures presented in the 2010 *Highway Capacity Manual* (HCM)⁶ for unsignalized intersections, was used to complete the level-of-service analyses.

Vehicle queue analyses are a direct measurement of an intersection's ability to process vehicles under various traffic control and volume scenarios and lane use arrangements. The vehicle queue analysis was performed using the *Synchro* intersection capacity analysis software which is based upon the methodology and procedures presented in the 2010 *Highway Capacity Manual*. The *Synchro* vehicle queue analysis methodology is a simulation based model which reports the number of vehicles that experience a delay of six seconds or more at an intersection. For unsignalized intersections, *Synchro* reports the 95th percentile vehicle queue. Vehicle queue lengths are a function of the capacity of the movement under study and the volume of traffic being processed by the intersection during the analysis period. The 95th percentile vehicle queue is the vehicle queue length that will be exceeded only 5 percent of the time, or approximately three minutes out of sixty minutes during the peak one hour of the day (during the remaining fifty-seven minutes, the vehicle queue length will be less than the 95th percentile queue length).

Analysis Results

Level-of-service and vehicle queue analyses were conducted for 2014 Existing, 2021 No-Build and 2021 Build conditions for the intersections within the study area. The results of the intersection capacity and vehicle queue analyses are summarized in Table 6, with the detailed analysis results presented in the Appendix.

Community Way at Cocasset Street

All movements exiting the Community Way approaches to Cocasset Street were shown to operate at LOS "B" or better during the peak hours under 2014 Existing and 2021 No-Build conditions. Under 2021 Build conditions, with the addition of Project-related traffic, motorists exiting the Community Way east connection to Cocasset Street were shown to operate at a LOS "C" during the peak hours with no residual vehicle queuing reported, indicating continued acceptable operating conditions at the intersections. All movements along Cocasset Street were shown to operate at LOS "A" under all analysis conditions with negligible vehicle queuing reported.

Community Way at the Project Site Roadway

All movements at the Project site roadway intersection with Community Way were shown to operate at LOS "A" under all analysis conditions with negligible vehicle queuing reported.

⁶Highway Capacity Manual, Transportation Research Board; Washington, DC; 2010.





Table 6 UNSIGNALIZED INTERSECTION LEVEL-OF-SERVICE AND VEHICLE QUEUE SUMMARY

		2014 E	Existing			2021 No	-Build		2021 Build					
Unsignalized Intersection/Peak Hour/Movement	Demand ^a	Delay ^b	LOSc	Queue ^d 95 th	Demand	Delay	LOS	Queue 95 th	Demand	Delay	LOS	Queue 95 th		
Cocasset Street at Community Way														
Weekday Morning:														
Cocasset Street EB LT/TH	187	0.0	A	0	208	0.0	A	0	212	0.2	A	0		
Cocasset Street WB TH/RT	437	0.0	A	0	472	0.0	A	0	472	0.0	A	0		
Community Way SWB LT/RT	3	11.2	В	0	3	11.6	В	0	16	11.8	В	0		
Weekday Evening:														
Cocasset Street EB LT/TH	490	0.1	A	0	530	0.1	A	0	541	0.2	A	0		
Cocasset Street WB TH/RT	288	0.0	A	0	318	0.0	A	0	318	0.0	A	0		
Community Way SWB LT/RT	1	9.9	A	0	1	10.1	В	0	7	10.3	В	0		
Cocasset Street at Community Way Extension														
Weekday Morning:														
Cocasset Street EB LT/TH	187	0.0	A	0	208	0.0	A	0	208	0.0	A	0		
Cocasset Street WB TH/RT	437	0.0	A	0	472	0.0	A	0	474	0.0	A	0		
Community Way Extension SB LT/RT	0	0.0	A	0	0	0.0	A	0	5	15.0	C	0		
Weekday Evening:														
Cocasset Street EB LT/TH	486	0.0	A	0	526	0.0	A	0	526	0.0	A	0		
Cocasset Street WB TH/RT	288	0.0	A	0	318	0.0	A	0	323	0.0	A	0		
Community Way Extension SB LT/RT	0	0.0	A	0	0	0.0	A	0	3	18.9	C	0		
Community Way at Community Way Extension and														
the Project Site Roadway Weekday Morning:														
Community Way EB LT/TH/RT	0	0.0	A	0	0	0.0	A	0	4	7.2	A	0		
Community Way WB LT/TH/RT	3	0.0	Α	0	3	0.0	A	0	3	0.0	Α	0		
Community Way Extension NB LT/TH/RT	0	0.0	Α	0	0	0.0	A	0	2	0.0	Α	0		
Project Site Roadway SB LT/TH/RT									18	8.4	A	0		
Weekday Evening:														
Community Way EB LT/TH/RT	4	0.0	A	0	4	0.0	A	0	15	5.3	A	0		
Community Way WB LT/TH/RT	1	0.0	A	0	1	0.0	A	0	1	0.0	A	0		
Community Way Extension NB LT/TH/RT	0	0.0	A	0	0	0.0	A	0	5	9.2	A	0		
Project Site Roadway SB LT/TH/RT									9	8.6	Α	0		

^aDemand in vehicles per hour.



^bAverage control delay per vehicle (in seconds).

^cLevel-of-Service.

^dQueue length in vehicles.

NB = northbound; SB = southbound; EB = eastbound; WB = westbound; SWB = southwestbound; LT = left-turning movements; TH = through movements; RT = right-turning movements.

SIGHT DISTANCE EVALUATION

Sight distance measurements were performed at the intersection of Community Way at the Project site roadway in accordance with MassDOT and American Association of State Highway and Transportation Officials (AASHTO)⁷ requirements. Both stopping sight distance (SSD) and intersection sight distance (ISD) measurements were performed. In brief, SSD is the distance required by a vehicle traveling at the design speed of a roadway, on wet pavement, to stop prior to striking an object in its travel path. ISD or corner sight distance (CSD) is the sight distance required by a driver entering or crossing an intersecting roadway to perceive an on-coming vehicle and safely complete a turning or crossing maneuver with on-coming traffic. In accordance with AASHTO standards, if the measured ISD is at least equal to the required SSD value for the appropriate design speed, the intersection can operate in a safe manner. Table 7 presents the measured SSD and ISD at the subject intersection.

Table 7
SIGHT DISTANCE MEASUREMENTS^a

	_	Feet	
Intersection/Sight Distance Measurement	Required Minimum	ISD ^b	Measured
Community Way at the Project Site Roadway			
Stopping Sight Distance:			
Community Way approaching from the east	200		255
Community Way/Cocasset Street approaching from the west	200		435
Intersection Sight Distance:			
Looking to the east from the Project Site Roadway	200	290/335	220^{c}
Looking to the west from the Project Site Roadway	200	290/335	270°

^aRecommended minimum values obtained from *A Policy on Geometric Design of Highways and Streets*, 6th Edition; American Association of State Highway and Transportation Officials (AASHTO); 2011; and based on a 30 mph approach speed on Community Way.

As can be seen in Table 7, the available lines of sight at the subject intersection were found to exceed or could be made to exceed the recommended minimum sight distance requirements to function in a safe manner based on a 30 mph approach speed along Community Way, a reasonable approach speed given that Community Way ends approximately 650 feet east of the Project site roadway.

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bValues shown are the intersection sight distance for a vehicle turning right/left exiting a roadway under STOP control such that motorists approaching the intersection on the major street should not need to adjust their travel speed to less than 70 percent of their initial approach speed.

^cWith the trimming and/or removal of trees and vegetation located along the north side of Community Way.

⁷A Policy on Geometric Design of Highways and Streets, 6th Edition; American Association of State Highway and Transportation Officials (AASHTO); 2011.

CONCLUSIONS

VAI has completed a detailed assessment of the potential impacts on the transportation infrastructure associated with the proposed construction of the Wyman Village residential community to be located off Community Way in Foxborough, Massachusetts. The following specific areas have been evaluated as they relate to the Project: i) access requirements; ii) potential off-site improvements; and iii) safety considerations; under existing and future conditions, both with and without the Project. Based on this assessment, we have concluded the following with respect to the Project:

- 1. Using trip-generation statistics published by the ITE⁸ for a similar land use as that proposed, the Project is expected to generate approximately 240 vehicle trips on an average weekday (two-way, 24-hour volume), with approximately 24 vehicle trips expected during the weekday morning peak-hour and 25 vehicle trips expected during the weekday evening peak-hour;
- 2. The Project will not have a significant impact (increase) on motorist delays or vehicle queuing over Existing or anticipated future conditions without the Project (No-Build conditions). Under all analysis conditions, all movements at the study intersections were shown to operate at LOS "C" or better with negligible vehicle queuing;
- 3. No apparent safety deficiencies were noted with respect to the motor vehicle crash history at the study intersection, which was found to have a motor vehicle crash rate <u>below</u> both the MassDOT statewide and District 5 average crash rate for an unsignalized intersection; and
- 4. Lines of sight at the Project site roadway intersection with Community Way were found to exceed or could be made to exceed the requirements to function in a safe manner based on a 30 mph approach speed along Community Way.

Based on the above, we have concluded that the Project can be accommodated within the confines of the existing transportation infrastructure in a safe and efficient manner with implementation of the recommendations that follow.

RECOMMENDATIONS

A detailed transportation improvement program has been developed that is designed to provide safe and efficient access to the Project site and address any deficiencies identified at off-site locations evaluated in conjunction with this study. The following improvements have been recommended as a part of this evaluation and, where applicable, will be completed in conjunction with the Project subject to receipt of all necessary rights, permits, and approvals.

Project Access

Access to the Project site will be provided by a new roadway that will intersect the north side of Community Way and will be aligned opposite the eastern connection between Community Way and Cocasset Street. The following recommendations are offered with respect to the design and operation of the Project site roadway:



⁸Ibid 2.

- ➤ The Project site roadway should be a minimum of 22-feet in width and accommodate two-way travel, with vehicles exiting the Project site at Community Way placed under STOP-sign control with a marked STOP-line provided.
- ➤ Vehicles approaching Community Way on the east extension between Cocasset Street and Community Way (opposite the Project site roadway) should be placed under STOP-sign control with a marked STOP-line provided.
- All signs and pavement markings to be installed within the Project site shall conform to the applicable standards of the *Manual on Uniform Traffic Control Devices* (MUTCD).⁹
- > Signs or landscaping along the Project site roadway internal to the Project site and at its intersection with Community Way should be designed and maintained so as not to restrict lines of sight.
- Existing vegetation located along the north side of Community Way in the vicinity of the Project site roadway and within the public right-of-way or on the Project site should be selectively trimmed or removed and maintained in order to provide the recommended lines of sight to and from the Project site roadway.

Off-Site

Independent of the Project, it is recommended that STOP-signs and marked STOP-lines be provided on the Community Way approaches to Cocasset Street (east and west branches).

With implementation of the above recommendations, safe and efficient access will be provided to the Project site and the Project can be constructed with minimal impact on the roadway system.

⁹Manual on Uniform Traffic Control Devices (MUTCD); Federal Highway Administration; Washington, D.C.; 2009.



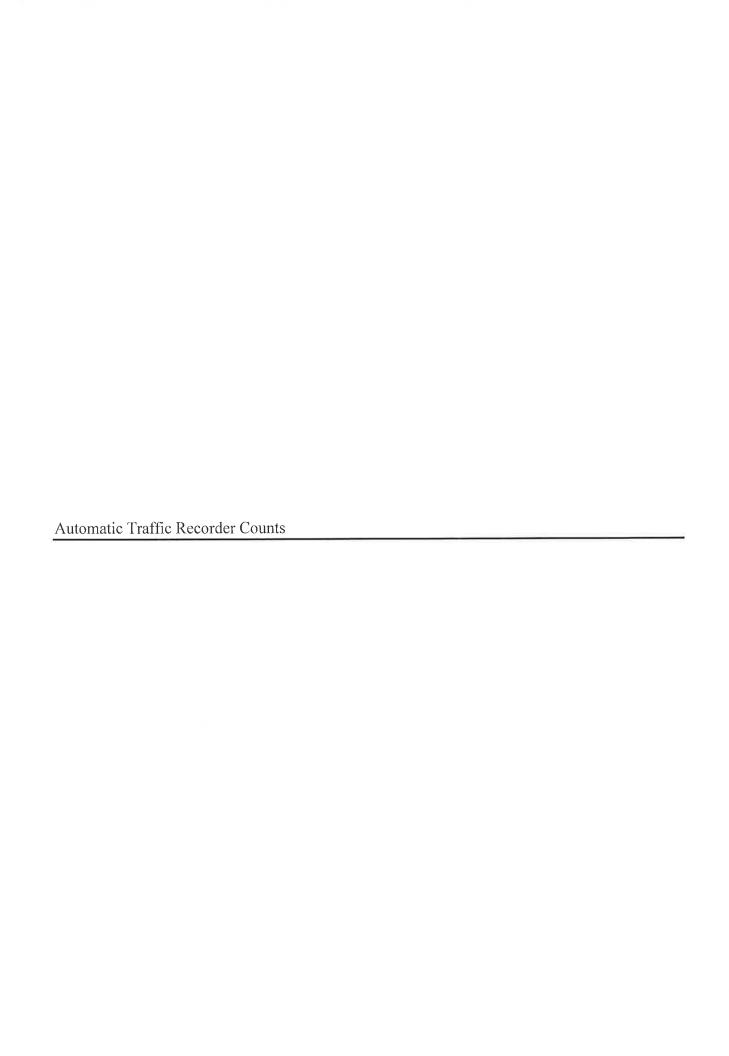
G:\6810 Foxborough, MA\Memos\Wyman Village Residential TIA 0814.docx

APPENDIX

TRAFFIC COUNT DATA
SEASONAL ADJUSTMENT DATA
MABELLE M. BURRELL ELEMENTARY SCHOOL TRAFFIC PROJECTIONS
VEHICLE TRAVEL SPEED DATA
MASSDOT CRASH RATE WORKSHEET
GENERAL BACKGROUND TRAFFIC GROWTH
BACKGROUND DEVELOPMENT BY OTHERS
TRIP-GENERATION CALCULATIONS
CAPACITY ANALYSIS WORKSHEETS

TRAFFIC COUNT DATA

Automatic Traffic Recorder Counts Manual Turning Movement Counts



Location: Cocasset Street Location: at Community Way City/State: Foxborough, MA

Site Code: 68100001 6810VOL1

ed Totals		Totals	Hour	В		Totals	Hour	VB	V	29-Jul-14	Start
Afternoor	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Tue	Time
				55	10			46	8		12:00
				54	13			49			12:15
				47	9	1		49	2 3 2		12:30
41.	57	222	42	66	10	190	15	46	2		12:45
				52	6		-	55	2		01:00
				50	0	- 1		41	3		01:15
				41	1	- 1		53	1		01:30
40	16	205	10	62		198	6	49	Ö		01:45
				45	2		Ü	46	3		02:00
				56	3 2 1			44	Ö		02:15
		4		53	2			49	3		02:30
42	14	226	6	72	1	203	8	64	2		02:45
			•	95	0	200	O	53	1		03:00
				97	1			57	0		03:15
		- 1		98	1	l l		71	2		03:30
64	7	385	4	95	2	264	3	83	0		03:45
٠.		000		87	4	204	0	67	4		04:00
				113	2	= {/		70	7		04:00
				106	3			72	12		04:13
68	48	407	11	101	2 4 2 3 2 3 6 5	278	37	69	14		04:45
50	10	10,		108	3	2,0	37	74	18		05:00
				136	6			76	35		05:00
				130	5			65	36		05:30
76	148	476	24	102	10	289	124	74	35		05:45
, 0	140	770	24	120	17	209	124	78	54		06:00
				111	25			62	75		06:00
				90	21			54	70		06:30
66	375	408	99	87	36	254	276	60	72 75		06:30
00	373	400	33	86	34	204	2/0		70		00.40
				85	52			42	76		07:00
				66	5Z			77	89		07:15
51	541	306	164	69	45 33	213	377	54 40	113 99		07:30
31	541	300	104	49	42	213	377		99		07:45
				49	47			44	88		08:00
				38	47			34	106		08:15
33	566	193	180	59	40	445	200	43	100 92		08:30
33	500	193	100	59	43	145	386	24	92		08:45
				49	45			26	74		09:00
				39	47			21	59		09:15
0.4	404	400	474	38	42		0.50	19	52		09:30
24	421	166	171	40	37	83	250	17	65		09:45
				27	36			15	57		10:00
				21	36	1		20	51		10:15
	0-0			16	40			10	50		10:30
13	352	87	149	23	37	51	203	6	45		10:45
				19	31			14	45		11:00
				23	34			9	49		11:15
22	2.5			26	49			4	35		11:30
11	340	79	162	11	48	33	178	6	49		11:45
536	2885			3160	1022			2201	1863		Total
65.0	35.0%			75.6%	24.4%			54.2%	45.8%		Percent

Location: Cocasset Street Location: at Community Way City/State: Foxborough, MA

ADT

ADT 8,360

Site Code: 68100001 6810VOL1

Start	30-Jul-14	V	/B		Totals		В	Hour	Totals	Combined Totals		
Time	Wed	Morning	Afternoon		Afternoon	Morning	Afternoon	Morning	Afternoon	Morning	Afternoor	
12:00		4	51			13	64					
12:15		3	61			7 9	77					
12:30		4	67			9	59					
12:45		6	57	17	236	6	58	35	258	52	49	
01:00		3	57			6	46					
01:15		4	51			3	49		1			
01:30		3	45			4	57					
01:45		4	56	14	209	2	64	15	216	29	42	
02:00		0	49		200	4	59					
02:00		3	55			4	49		- 1			
02:30		2	60			1	63		1			
02:30		0	58	5	222	Ó	64	9	235	14	45	
03:00		1	47	5	222	0	76	3	200	17	40	
03:00		0	55			3	86		- 1			
03:15						2						
03:30		1	82 87	E	271	0	99	5	351	10	62	
03:45		3		5	2/1			3	331	10	02	
04:00		2	74			0	128					
04:15		4	66				108					
04:30		16	78	00	000	3	94	0	400	27	7	
04:45		7	75	29	293	4	100	8	430	37	7:	
05:00		19	75			2	120					
05:15		37	73			3	112					
05:30		41	74			7	127				_	
05:45		38	74	135	296	6	91	18	450	153	7.	
06:00		47	50			14	110					
06:15		71	63			23	95					
06:30		79	53			30	99					
06:45		67	55	264	221	37	92	104	396	368	- 6	
07:00		68	53			43	89					
07:15		72	52			46	69					
07:30		87	46			36	65					
07:45		101	44	328	195	37	82	162	305	490	5	
08:00		99	39			48	58					
08:15		97	39			46	61					
08:30		97	36			49	41					
08:45		108	25	401	139	42	46	185	206	586	3	
09:00		73	35			47	58					
09:15		50	21			46	40					
09:30		70	23			41	36					
09:45		69	17	262	96	41	41	175	175	437	2	
10:00		59	27	202	00	31	30	1,70	.,,		_	
10:15		57	16			47	38					
10:30		47	17			44	34					
10:30		39	21	202	81	42	21	164	123	366	2	
11:00		43	7	202	01	47	35	107	120	500	-	
11.00		43	/ 0			38	23					
11:15		42	8				25					
11:30		49	7	405	00	61	25	100	105	393	1	
11:45		61	7	195	29	52	22	198	105			
Total		1857	2288			1078	3250			2935	55	
Percent		44.8%	55.2%			24.9%	75.1%			34.6%	65.4	
Grand Total		3720				2100	6410			5820		
Percent		45.3%	54.7%			24.7%	75.3%			34.8%	65.2	

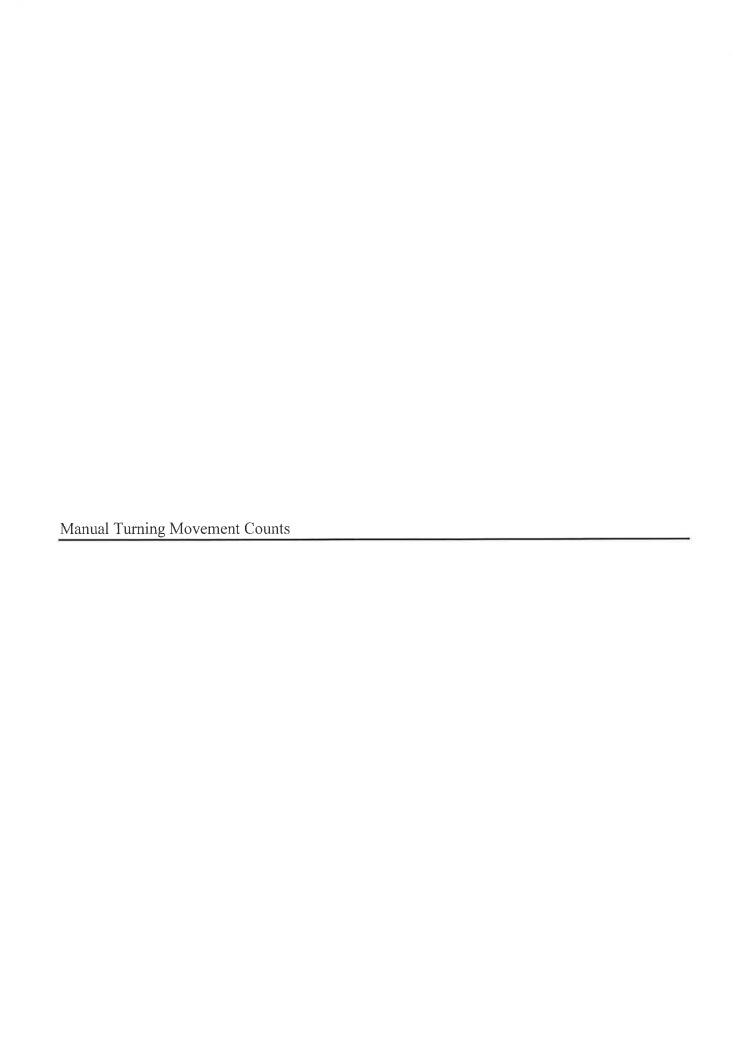
AADT 8,360

Site Code: 68100001 6810VOL1

Accurate Counts 978-664-2565

Location: Cocasset Street
Location: at Community Way
City/State: Foxborough, MA

Week Average WB EB	16 38	10 12	8	4	33 10	130 21		_													142 200					- 1	08:00 08:00			292 463	8358	156 < school	すいの
Sun Sun WB EB \	k	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*:	0 0	0	0				0		
Sat WB EB	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	* 5	*	*	*	*	*	*	*	0	0	100	*		2 2 2 2 2 3	0		
Fri WB EB	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	0	0	r	¥.	4		0		
Thu WB EB		(#		*	*	*	*	•	*	*	*	*	*	k	*	*	*	*	*	*	*	*	*	*	0	0	,	*			0		
Wed FB	17 35	14 15	2	5		135 18															139 206				:,7:	473	08:00 11:00		17:00 17:00		8473		
Tue WB FB	l LC	9	80	3	37 11	124 24															145 193					246			17:00 17:00	289 476	8246)	AADT 8,360
28-Jul-14 WR FR		*	*	*	*	*	*	*	*	*	*	*	*	*	*	1.00	*	*	•	*	*	*	*	*	0	0					C	ò	ADT 8,360
Start	12:00 AM	01:00	00.20	03:00	04:00	02:00	00:90	02:00	08:00	00:60	10:00	11:00	12:00 PM	01:00	02:00	03:00	04:00	02:00	00:90	00:20	08:00	00:60	10:00	11:00	Lane	Dav	AM Peak	Vol	PM Peak	Vol	Сотр.	Total	ADT



N/S Street: Community Way E/W Street: Cocasset Street
City/State: Foxborough, MA
Weather: Cloudy

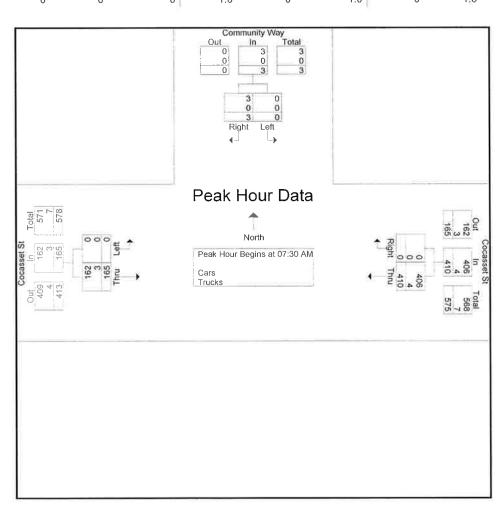
File Name : 68100001 Site Code : 68100001 Start Date 7/29/2014
Page No 1

		Cocasset St From West	1	Cocasset St From East		Community W From North	
Int. Total	Thru	Left	Right	Тһги	Right	Left	Start Time
118	38	0	0	78	2	0	07:00 AM
144	53	0	0	90	1	0	07:15 AM
159	43	0	0	115	1	0	07:30 AM
131	35	0	0	95	1	0	07:45 AM
552	169	0	0	378	5 ·	0	Total
131	38	0	0	92	1 :	0	08:00 AM
157	49	0	0	108	0	0	08:15 AM
128	46	0	0	82	0	0	08:30 AM
147	43	0	0	103	1	0	08:45 AM
563	176	0	0	385	2	0	Total
1115	345	0	0	763	7	0	Grand Total
	100	0	0	100	100	0	Apprch %
	30.9	0	0	68.4	0.6	0	Total %
1100	339	0	0	754	7	0	Cars
98.7	98.3	0	0	98.8	100	0	% Cars
15	6	0	0	9	0	0	Trucks
1.3	1.7	0	0	1.2	0	0	% Trucks

N/S Street : Community Way E/W Street : Cocasset Street City/State : Foxborough, MA Weather : Cloudy

File Name : 68100001 Site Code : 68100001 Start Date : 7/29/2014 Page No : 2

	Со	mmunity Wa	У	(Cocasset St			Cocasset St		
		From North			From East	- 6		From West		
Start Time	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App Total	Int. Total
Peak Hour Analysis From 07:0	00 AM to 08:4	45 AM - Peak	(1 of 1							
Peak Hour for Entire Interse	ction Begins	s at 07:30 A	M							
07:30 AM	0	1	1	115	0	115	0	43	43	159
07:45 AM	0	1	1	95	0	95	0	35	35	131
08:00 AM	0	1	1	92	0	92	0	38	38	131
08:15 AM	0	0	0	108	0	108	0	49	49	157
Total Volume	0	3	3	410	0	410	0	165	165	578
% App. Total	0	100		100	0		0	100	į	
PHF	.000	.750	.750	.891	.000	.891	.000	.842	.842	.909
Cars	0	3	3	406	0	406	0	162	162	571
% Cars	0	100	100	99.0	0	99.0	0	98.2	98.2	98.8
Trucks	0	0	0	4	0	4	0	3	3	7
% Trucks	0	0	0	1.0	0	1.0	0	1.8	1.8	1.2



N/S Street: Community Way E/W Street: Cocasset Street City/State: Foxborough, MA Weather: Cloudy

File Name : 68100001 Site Code : 68100001 Start Date 7/29/2014 Page No 7

				s Printed- Trucks	Group		
		Cocasset St From West		Cocasset St From East		Community W From North	
Int. Total	Thru	Left	Right	Thru	Right	Left	Start Time
0	0	0	0	0	0	0	07:00 AM
1	1	0	0	0	0	0	07:15 AM
3	1	0	0	2	0	0	07:30 AM
3	1	0	0	2	0	0	07:45 AM
7	3	0	0	4	0	0	Total
0	0	0	0	0	0	0	08:00 AM
1	1	0	0	0	0	0	08:15 AM
2	1	0	0	1	0	0	08:30 AM
5	1	0	0	4	0	0	08:45 AM
8	3	0	0	5	0	0	Total
15	6	0	0	9	0	0	Grand Total
	100	0	0	100	0	0	Apprch %
	40	0	0	60	0	0	Total %

N/S Street: Community Way E/W Street: Cocasset Street
City/State: Foxborough, MA
Weather: Cloudy

Grand Total

Apprch %

Total %

0

0

0

0

3

8

100

72.7

File Name 68100001 Site Code :: 68100001 Start Date : 7/29/2014

Page No 10

		munity Way om North			ocasset St rom East			ocasset St rom West				
Start Time	Left	Right	Peds !	Thru .	Right	Peds	Left	Thru	Peds	Exclu. Total	Inclu Total	Int. Total
07:00 AM	0	0	0	2	0	0	0	0	0	0	2	2
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0
07:30 AM	0	0	0	1	0	0	0	0	0	0	1	1
07:45 AM	0	0	1	2	0	0	0	0	0	1	2	3
Total	0	0	1	5	0	0	0	0	0	1	5	6
08:00 AM	0	0	1	1	0	0	0	0	0	1	1	2
08:15 AM	0	0	0	2	0	0	0	1	0	0	3	3
08:30 AM	0	0	0	0	0	0	0	0	0	0	0	C
08:45 AM	0	0	1	0	0	0	0	2	0	1	2	3
Total	0	0	2	3	0	0	0	3	0	2	6	8

0

0

0

0

3

100

27.3

0

3

21.4

11

78.6

14

N/S Street : Community Way E/W Street : Cocasset Street City/State : Foxborough, MA Weather : Cloudy

File Name: 68100001 Site Code: 68100001 Start Date : 7/29/2014

Page No : 1

Groups Printed- Cars - Trucks

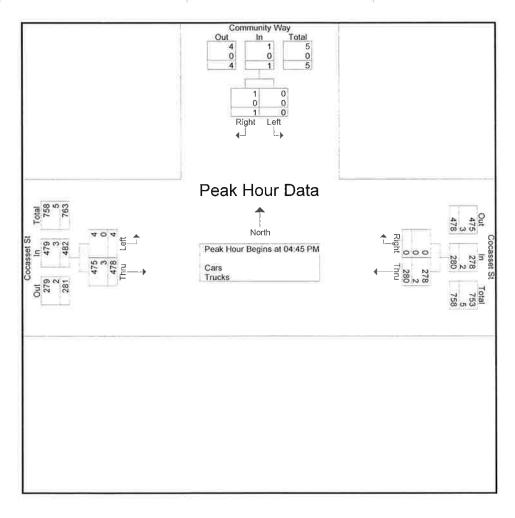
		Cocasset St From West		Cocasset St From East		Community Wa From North			
Int. Total	Thru	Left	Right	Thru	Right	Left	Start Time		
160	97	0	0	60	3	0	04:00 PM		
187	107	3	0	77	0 .	0	04:15 PM		
188	122	1	0	64	0	1	04:30 PM		
174	98	1	0	75	0	0	04:45 PM		
709	424	5	0	276	3	1	Total		
170	103	0	0	67	0	0	05:00 PM		
224	152	2	0	69	1	0	05:15 PM		
195	125	1	0	69	0	0	05:30 PM		
170	90	1	0	74	4 :	1	05:45 PM		
759	470	4	0	279	5	1	Total		
1468	894	9	0	555	8	2	Grand Total		
	99	1	0	100	80	20	Apprch %		
	60.9	0.6	0	37.8	0.5	0.1	Total %		
1454	888	9	0	547	8	2	Cars		
99	99.3	100	0	98.6	100	100	% Cars		
14	6	0	0	8	0	0	Trucks		
1	0.7	0	0	1.4	0	0	% Trucks		

N/S Street: Community Way E/W Street: Cocasset Street City/State : Foxborough, MA Weather : Cloudy

File Name: 68100001 Site Code : 68100001 Start Date : 7/29/2014

Page No : 2

		mmunity Wa From North	y		Cocasset St From East		Cocasset St From West			
Start Time	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	Int. Total
Peak Hour Analysis From 04:0	00 PM to 05:4	5 PM - Peak	1 of 1							
Peak Hour for Entire Interse	ection Begins	at 04:45 P	M							
04:45 PM	0	0	0	75	0	75	1	98	99	174
05:00 PM	0	0	0	67	0	67	0	103	103	170
05:15 PM	0	1	1	69	0	69	2	152	154	224
05:30 PM	0	0	0 -	69	0	69	1	125	126	195
Total Volume	0	1	1	280	0	280	4	478	482	763
% App. Total	0	100		100	0		0.8	99.2		
PHF	.000	.250	.250	.933	.000	.933	.500	.786	.782	.852
Cars	0	1	1	278	0	278	4	475	479	758
% Cars	0	100	100	99.3	0	99.3	100	99.4	99.4	99.3
Trucks	0	0	0	2	0	2	0	3	3	5
% Trucks	0	0	0	0.7	0	0.7	0	0.6	0.6	0.7



N/S Street : Community Way E/W Street : Cocasset Street City/State : Foxborough, MA Weather : Cloudy

File Name: 68100001 Site Code : 68100001 Start Date : 7/29/2014

Page No : 7

Groups Printed- Trucks

	Community V From North			Cocasset St From West			
Start Time	Left	Right	Thru!	Right	Left	Thru	Int. Total
04:00 PM	0	0	2	0	0	2	4
04:15 PM	0	0	1	0	0	0	1
04:30 PM	0	0	2	0	0	1	3
04:45 PM	0	0	0	0	0	2	2
Total	0	0	5	0	0	5	10
05:00 PM	0	0	0	0	0	1	1
05:15 PM	0	0	0	0	0	0	0
05:30 PM	0	0	2	0	0	0	2
05:45 PM	0	0	1	0	0	0	1
Total	0	0	3	0	0	1	4
Grand Total	0	0	8	0	0	6	14
Apprch %	0	0	100	0	0	100	
Total %	0	0	57.1	0	0	42.9	

N/S Street : Community Way E/W Street : Cocasset Street City/State : Foxborough, MA Weather : Cloudy

Community Way

File Name: 68100001 Site Code : 68100001 Start Date : 7/29/2014 Page No : 10

Groups Prin	ted- Bikes	Peds		
Cocasset St From East			ocasset St om West	
Right	Peds	Left	Thru	Ped

	Fi	rom North		From East			From West					
Start Time	Left	Right	Peds	Thru	Right	Peds	Left	Thru	Peds	Exclu. Total	Inclu. Total	Int. Total
04:00 PM	0	0	0	0	0	0	0	0	0	0	0	0
04:15 PM	0	0	0	0	0	0	0	0	0	0	0	0
04:30 PM	0	0	0	1	0	0	0	1	0	0	2	2
04:45 PM	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	1	0	0	0	1	0	0	2	2
05:00 PM	0	0	0	2	0	0	0	2	0	0	4	4
05:15 PM	0	0	0	1	0	0	0	0	0	0	1	1
05:30 PM	0	0	0	1	0	0	0	0	0	0	1	1
05:45 PM	0	0	0	0	0	0	0	1	0	0	1	1
Total	0	0	0	4	0	0	0	3	0	0	7	7
Grand Total	0	0	0	5	0	0	0	4	0	0	9	9
Apprch %	0	0		100	0		0	100				
Total %	0	0		55.6	0		0	44.4		0	100	



MASSACHUSETTS HIGHWAY DEPARTMENT - STATEWIDE TRAFFIC DATA COLLECTION

2011 WEEKDAY SEASONAL FACTORS *

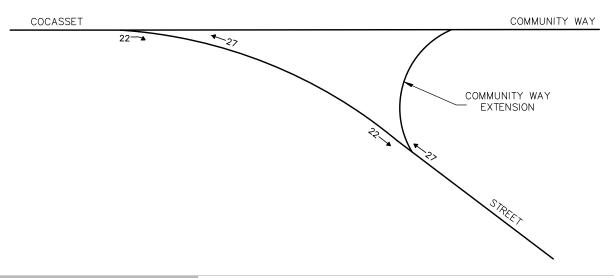
Noie: These are weekday factors. The average of the factors for the year will not equal 1, as weekend data are not considered.

FACTOR GROUP	JAN	FEB	MAR	APR	MAY	NOF	JUL	AUG	SEP	i oci	AON	DEC
COCIO - WEST INTERSTATE	0.98	0.93	0.90	0.89	0.90	0.88	0.91	0.90	0.89	0.89	0.93	0.95
CENTRAL MAINE COLLECTOR (R-5)	1.12	1.12	1.07	0.99	0.91	06.0	0.86	0.86	0.92	0.93	1.01	1.05
GROUP 3A - RECREATIONAL **(1.4) See below	126	1.25	1.20	1.06	96.0	0.89	0.76	0.76	0.92	0.99	1.08	1.14
GROUP 3B - RECREATIONAL ***(5) See below	1.22	1.26	1.22	1.06	0.96	0.90	0.72	0.74	0.97	1.02	1.14	1.15
GROID 4 - 1-495 INTERSTATE	1.02	1.00	1.00	0.96	0.92	0.89	0.85	0.83	0.93	0.96	1.01	1.03
GROUP 5 - FAST INTERSTATE	1.04	1.00	96.0	0.93	0.92	0.91	0.91	0.89	0.93	0.93	96.0	1.01
GROUP 6 - URBAN ARTERNIS, COLLECTORS & RURAL ARTERNIS (R-2. R-3)	1.03	1.01	0.96	0.92	0.91	0.90	0.92	0.92	0.93	0.92	0.97	76.0
GROUP 7 - 1-84 PROXIMITY (STAS, 17,3921)	1.24	1.24	1.15	1.04	0.99	1.00	0.93	0.89	1.05	1.05	1.05	1.12
GROUP 8 - 1-295 PROXIMITY (STA, 6590)	1.00	0.99	0.95	0.92	0.94	0.91	0.93	0.92	0.95	0.94	0.97	0.95
GROUP 9 - 1-195 PROXIMITY (STA. 7)	1.13	1.05	1.03	0.95	0.89	0.87	0.86	0.79	0.88	0.91	0.99	1.03
RECREATIONAL: (ALI YEARS)		2011	AXLEC	ORREC	TION F	2011 AXLE CORRECTION FACTORS				ROUND OFF	O OFF	
™GROUP 3A: 1. CAPE COB (ALL TOWNS) 2.PLYMOUTH(SOUTH OF RTE.3A)		ROAI FI	ROAD INVENTORY FUNCTIONAL CLASSIFICATION	NTORY NAL ATION	8	AXI.E CORRECTION FACTOR	Š "			0 - 99910 > 1,000100	100	
7014, 7079,7080,7080,7091,7082,7093,7094,7095,7095,7097,7109,7178 3.MARTHAS VINEYARD			5 1 1 1			0.95					20	
4.NANTUCKET			3 0,5,6			0.98						
			, 1			96.0						
**GROUP 3B:			0 10			0.98						
5.PERMANENTS 2 & 189			ເດ			0.98						
1093,1094,1095,1096,1097,1098,1109,1100,1101,1102,1104,			9,0			0.99						
1105,1106,1107,1108,1113,1114,1116,2196,2197,2198		Appl	Apply I-84 factor to stations: 3290,3929	or to stati	oris: 3290	3929						



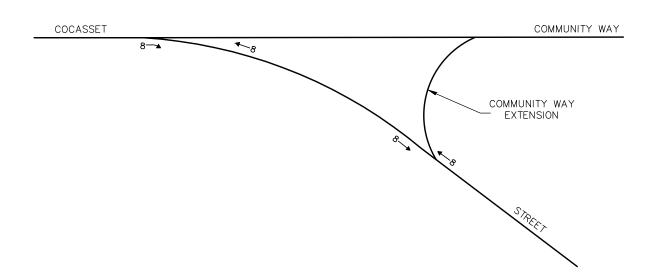
WEEKDAY MORNING PEAK HOUR





WEEKDAY EVENING PEAK HOUR







Mabelle M. Burrell School Peak Hour Traffic Volumes

Figure A-1

Institute of Transportation Engineers (ITE) Trip Generation, 9th Edition Land Use Code (LUC) 520 - Elementary School

Average Vehicle Trips Ends vs: Students Independent Variable (X): 367

AVERAGE WEEKDAY DAILY

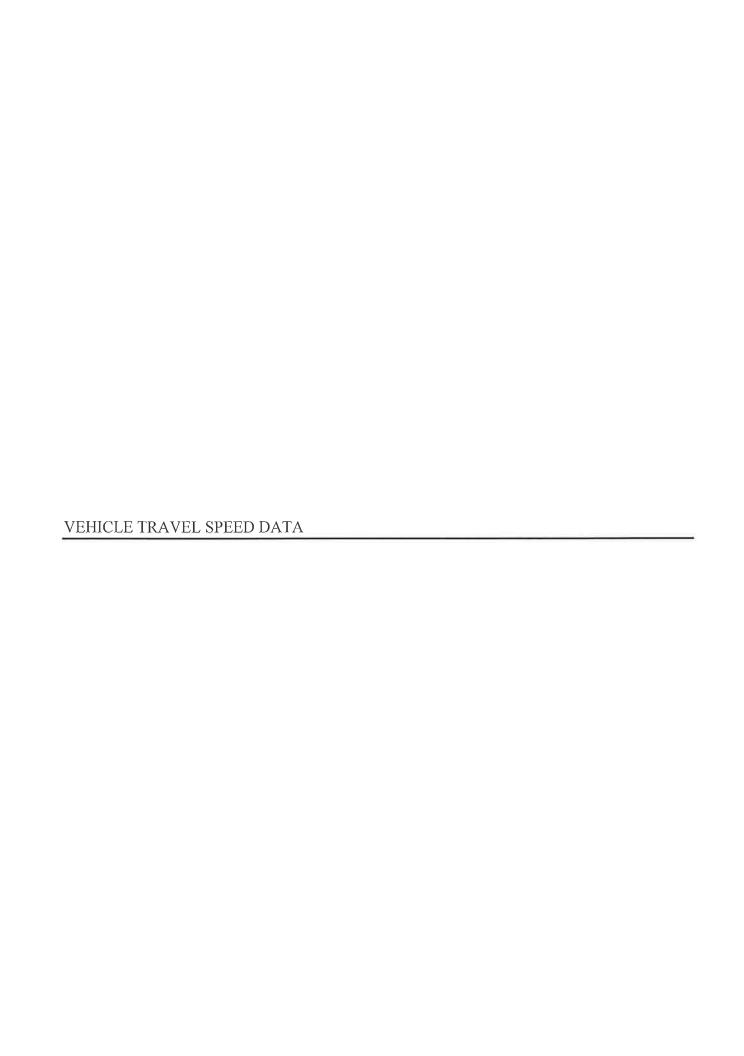
T = 1.29 * (X) T = 1.29 * 367 T = 473.43 T = 474 vehicle trips with 50% (237 vph) entering and 50% (237 vph) exiting.

WEEKDAY MORNING PEAK HOUR

T = 0.45 * (X) T = 0.45 * 367 T = 165.15 T = 165 vehicle trips with 55% (91 vph) entering and 45% (74 vph) exiting.

WEEKDAY EVENING PEAK HOUR OF ADJACENT STREET TRAFFIC

T = 0.15 * (X)T = 0.15 * 367T = 55.05T = 55 vehicle tripswith 49% (27 vph) entering and 51% (28 vph) exiting,



Accurate Counts 978-664-2565

Location: Cocasset Street
Location: at Community Way
City/State: Foxborough, MA
Westbound

_	ant Percent	40 46	50 53	45 47	42 44	44 47	45 48	43 46	42 45	40 43	42 46				42 45		42 44	42 45	42 45	42 44	42 45	40 44	41 44	40 44	41 47					
86	Percen																													
	Total	15	9	00	က	37	124	276	377	386	250	203	178	190	198	203	264	278	289	254	213	145	83	51	33	4064		08:00	386	17:00
9/	666	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	%0.0			
71	75	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	%0.0			
99	20	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	%0.0			
61	65	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	%0.0			
56	90	0	0	0	0	0	0	-	0	0	0	0	0	0	0	0	_	0	0	0	0	0	0	0	0	2	%0.0	00:90	÷	15:00
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41	45	-	0	ď	· -	- ==	4	77	86	62	20	32	34	37	44	54	62	99	63	52	46	19	1	7	က	874	21.5%	02:00	98	16.00
	40								183																				189	17:00
									72																				106	15.00
26	30	2	ı c	· C	o c	o -		, 4-	- ∞	23	ر ا	<u> </u>) <u>/</u>	4	4	m	4	. 60	· (C	4	. ~	1 4	4	· (*)	9	113	2.8%	08:00	23	16.00
21	25	С	0 0	· C	o c) C	· -	- c	0 0	0 0	1 00) -	· c) C	0 0	o C	2	ı c	0 0	· ~			· C	o	0	15	0.4%	00:60	9	15.00
16	20	C	0	· C	0 C	0 0) C	o c	· —			0 0	0 0	o C	o	o C	o	o C	· -	٠ ٢		0 0) C	o C	0	4	0.1%	07:00	•	17.00
	15	C	0 0	· C	0 0	0 0	~	- c	0 0	o C	0 0) (- c	o	o C) C	0 C) C	m) C	o c	o c	o c) C	0	. 12	0.1%	02:00	-	17.00
Start	Time	07/29/14	01.00	00.00	02:00	03:00	05:00	06.00	02:00	00:30	00:00	10.00	11.00	10 PM	13.00	14:00	15:00	16:00	17:00	18.00	19:00	20:00	21.00	22:00	23.00	Total	Percent	AM	Veak	PM

Accurate Counts 978-664-2565

Location : Cocasset Street
Location : at Community Way
City/State: Foxborough, MA

Westbound				Action Company	-				I			00	1	-		1,100	147.0
Start	-	16	21	56	31	36	41			99	61	99	- 1	9/		uics	LICE
Time	15	20	25	30	35	40	45			09	65	20	75	666	Total	Percent	Percent
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01.00	o C			· -	ıc.	C.	m			0	0	0	0	0	14	45	48
00:00	o c	o c	o C	- c) C	00	0			-	С	0	0	0	Ŋ	56	58
03.00	o c	0 0	0 0	· -	· ←	10	ı C			C	С	0	0	0	c)		48
03:00	o c	o c	o c			10	000			c	0	0	0	0	29		45
04:00	o c	o c	o	ס גמ	20	23.0	9 4			0	0	0	0	0	135		46
06:00		o C) C	o (C	4 4	127	92			0	0	0	0	0	264		45
02:00	0 0	o) C	0 0	. 4	182	95			0	0	0	0	0	328		45
00:70	o c	o c	· -	1 6	- 65	204	84			0	0	0	0	0	401		44
00:00	0 0	o c		2 ~	57	139	50			0	0	0	0	0	262		45
10:00	o	o C) C		50	100	37			0	0	0	0	0	202		45
11:00	o) C	o	. ~	27.0	108	26			0	0	0	0	0	195		44
12 PM	o C	o C	· -	l m	55	123	49			0	0	0	0	0	236		44
13.00	0 0	0	0	מו	52	118	29			0	0	0	0	0	209		44
14:00	0	0	7	18	51	102	45			0	0	0	0	0	222		44
15:00	0	0	_	13	54	140	22			0	0	0	0	0	271		44
16:00	· -	0	0	4	45	162	69			0	0	0	0	0	293		45
17:00	0	0	-	4	52	154	81			0	0	0	0	0	296		44
18:00	0	0	0	_	27	109	71		_	0	0	0	0	0	221	43	46
19:00	0	0	0	2	99	86	34			0	0	0	0	0	195		44
20:00	0	0	0	4	40	74	9			0	0	0	0	0	139	40	44
21:00	0	0	2	4	31	47	0			0	0	0	0	0	96		44
22.00	C	0	0	က	56	37	13			0	0	0	0	0	81		44
23:00	0	0	0	0	ဖ	18	m			0	0	0	0	0	29		46
Total	2	0	00	114	885	2106	808			_	0	0	0	0	4145		
		%0.0	0.2%	2.8%	21.4%	20.8%	21.9%	2.7%	0.2%	0.0%	%0.0	0.0%	0.0%	%0.0			
	02:00		08:00	08:00	08:00	08:00	00:20		02:00	02:00					08:00		
reak Vol	-		-	16	91	204	92	11	*-	_					401		
	16:00		14:00	14:00	19:00	16:00	17:00	18:00	16:00						17:00		
Yeak Vol	~		2	00	99	162	8	12	_						296		
Grand	7	4	23	227	1740	4148	1782	258	17	က	0	0	0	0	8209		
l otal Percent	0.1%	%0.0	0.3%	2.8%	21.2%	50.5%	21.7%	3.1%	0.2%	%0.0	%0*0	%0.0	%0.0	%0.0			
		15 50 85	15th Percentile 50th Percentile 85th Percentile	 <u>a a a a</u>	32 MPH 37 MPH 42 MPH												
		CD CD	95th Percentile	 <u>a</u>	45 VIVI												

34-43 MPH 10 MPH Pace Speed Statistic s

Number in Pace:
Percent in Pace:
Number of Vehicles > 40 MPH:
Percent of Vehicles > 40 MPH:
Mean Speed(Average):

5908 72.0% 2060 25.1% 38 MPH

Accurate Counts 978-664-2565

Location: at Community Way

Location : Cocasset Street

Site Code: 68100001 6810SPD1 Percent 08:00 17:00 476 180 9666 - tr 0.0% 00:20 00:60 08:00 10:00 04:00 15:00 18:00 1 0 0 0 22 0.5% 10:00 City/State: Foxborough, MA 00:90 15:00 07/29/14 01:00 02:00 03:00 04:00 05:00 07:00 08:00 09:00 11:00 12:00 15:00 16:00 17:00 18: Eastbound Start

Accurate Counts 978-664-2565

Location: Cocasset Street Location: at Community Way City/State: Foxborough, MA

85th Percent 11:00 17:00 450 0 %0.0 %0.0 06:00 %0.0 0.0% 22:00 %0.0 0.0% 02:00 0.1% 15:00 1.5% 08:00 15:00 1.5% 131 00:20 16:00 19.9% 93 11:00 4835 2480 57.3% 17:00 56.8% 303 19.3% 33 MPH 37 MPH 41 MPH 44 MPH 6486 76.2% 1841 21.6% 38 MPH 34-43 MPH 19:00 1645 10:00 18.7% 00:60 12:00 90000000044**r**44**6**044004r44800 9 Number in Pace:
Percent in Pace:
Number of Vehicles > 40 MPH:
Percent of Vehicles > 40 MPH:
Mean Speed(Average): 15th Percentile: 50th Percentile: 85th Percentile: 95th Percentile: 10 MPH Pace Speed : 0.2% 0.2% 18:00 19 04:00 18:00 10:00 0.4% 4 0.3% 37 0.1% 00:90 0.2% 07/30/14 01:00 02:00 03:00 04:00 05:00 06:00 07:00 08:00 10:00 15:00 17:00 17:00 18:00 20:00 21:00 22:00 23:00 Total Percent AM Peak Vol. Statistic **∃astbound** 11:00 12 PM 13:00 Percent Time Start

Accurate Counts 978-664-2565

State: FO	CITV/State: roxborough, MA	5														,	COLOSED
estbound, E	Westbound, Eastbound		20	90	4	ď	41	46	r.	Υ.	0	99	7.1	76		85th	95th
Jimo	- 1	2 - 2	- 7 - 7	0 7 0 %	ر د بر	80	- 4 - 7	50	5.5	90	- K	20	7.5	666	Total	Percent	Percent
07/29/14	2 0	07	3 C	3 0	10 8	29	5. 5.	2 2	5	0	0	0	0	0	57	42	46
01.00	0	0 0	o C	1 0		0		ı 	-	0	0	0	0	0	16	42	20
02:00	0	0	0	0	· ←	cy o	. 9	-	_	0	0	0	0	0	14	45	51
03:00	0	0	0	0	2	m	7	0	0	0	0	0	0	0	_	42	44
04:00	0 0	0	· —	5	ı 0	19	13	4	0	0	0	0	0	0	48	43	46
05:00	· -) ~	. 2	0	12	68	46	18	0	0	0	0	0	0	148	44	47
00.90	. ~	2	ı -	4	09	179	111	15	0	_	0	0	0	0	375	43	45
07:00	10	2 1	0	ത	88	264	155	20	2	-	0	0	0	0	541	43	45
08:00) C	lα	۰ ۸	24	152	275	102	∞		0	0	0	0	0	999	4	44
00.00	· -) ~	9	17	94	204	75	22		0	0	0	0	0	421	42	45
10:00	· 67	4	_	4	80	177	59	9		0	0	0	0	0	352	4	44
11:00	· -	_	-	6	80	171	67	တ	-	0	0	0	0	0	340	4	44
12 PM	-	. C	-	6	87	223	80	o		0	0	0	0	0	412	4	44
13.00	· C	o a	-	00	85	213	98	10		0	0	0	0	0	403	45	44
14.00	o C	· C	_	4	95	227	86	4		0	0	0	0	0	429	41	44
15.00	0 0) +-	4	7	125	378	115	12		_	0	0	0	0	649	41	44
16.00	ıc	-	С	6	117	404	141	1		0	0	0	0	0	685	41	44
17.00	· (*)	. (*)	0	7	142	410	176	22		0	0	0	0	0	765	42	45
18:00	· 	, ru	٥ ر	· rc	109	394	137	o		0	0	0	0	0	662	41	44
19.00		· -	1 ~	, ro	116	288	96	9		0	0	0	0	0	519	41	44
20:00	· a	-	,	9	92	192	43	ო	0	0	0	0	0	0	338	40	43
21:00	0	0	0	6	68	129	39	4	0	0	0	0	0	0	249	41	4
22:00	0	0	0	4	33	80	16	2	ო	0	0	0	0	0	138	40	4
23.00	O	0	0	00	19	22	19	œ		0	0	0	0	0	112	42	47
Total	16	26	25	166	1689	4397	1696	210		m	0		0	0	8246		
Percent	0.2%	0.3%	0.3%	2.0%	20.5%	53.3%	20.6%	2.5%	0.2%	%0.0	0.0%	0.0%	0.0%	%0.0			
AM	10:00	10:00	00:60	08:00	08:00	08:00	00:20	00:60	00:20	00:90					08:00		
Vol.	က	4	9	24	152	275	155	22	2	-					999		
PM C	17:00	18:00	15:00	15:00	17:00	17:00	17:00	17:00	22:00	15:00					17:00		
בים בי																	

Accurate Counts 978-664-2565

Location : at Community Way

Location : Cocasset Street

Site Code: 68100001 6810SPD1 85th Percent Total 529 144 100 346 336 3366 3366 22 723 723 723 746 500 3473 8473 8473 8473 08:00 17:00 746 0 %0.0 00:90 %0.0 %0.0 0.0% 22:00 %0.0 %0.0 02:00 %0.0 0.2% 05:00 15:00 00:60 2.1% 18:00 17:00 21.0% 00:20 165 20.8% 141 4586 54.1% 08:00 17:00 8983 457 74.1% 3901 23.3% 38 MPH 20.2% 32 MPH 37 MPH 41 MPH 44 MPH 12394 34-43 MPH 3385 08:00 19:00 20.0% 180 08:00 14:00 20 22 346 Number in Pace:
Percent in Pace:
Number of Vehicles > 40 MPH:
Percent of Vehicles > 40 MPH:
Mean Speed(Average): 50th Percentile: 85th Percentile: 95th Percentile: MPH Pace Speed : Percentile 0.3% 0.2% 04:00 14:00 3 42 15th 10:00 18:00 0.2% 4 10 City/State: Foxborough, MA Westbound, Eastbound 16:00 05:00 21 07/30/14 01:00 02:00 03:00 04:00 06:00 07:00 08:00 09:00 13:00 14:00 15:00 17:00 17:00 18:00 18:00 19:00 22:00 22:00 22:00 22:00 22:00 Percent Peak Vol. Peak 11:00 12 PM Statistic Start Time





CRASH RATE WORKSHEET

CITY/TOWN:	Foxborough		•	COUNT DA	TE:	2014	MHD USE ONLY
DISTRICT: 5	UNSIGN	ALIZED: [Yes	SIGNA	LIZED :		Source #
		~ IN ⁻	TERSECTIO	N DATA ~	•		
MAJOR STREET :	Cocasset S						ST#
MINOR STREET(S)	Community	Way					ST#
						3	ST#
	8						ST#
	3*						ST#
	_						
INTERSECTION	North		1	1			INTERSECTION
DIAGRAM	IVOITI		2	4	288		REF#
(Label Approaches)		-	~>	,			
		490	3				
			1876	0			
			Deak Hay	. Valumas			
APPROACH:	1	2	3	r Volumes 4	5	Total	
DIRECTION:	NB	SB	EB	WB		Entering Vehicles	
VOLUMES (AM/PM)	0	1	490	288		779	
"K" FACTOR:	0.090	APPROA	CH ADT :	8,656	ADT = TOTA	L VOL/"K" FACT.	
TOTAL # OF		# OF		AVERA	GE#OF	0.00	
ACCIDENTS:	1	YEARS:	5	ACCIDE	NTS(A):	0.20	
CRASH RATE CALC	ULATION:	0.06	RATE =	(A * 1,	000,000) 「* 365)		
Comments : Crash rat	te is significant	if > 0.50 oreahe	s nor may for				
	7 crashes per m						



Proposed Wyman Village Residential Community Foxborough, MA

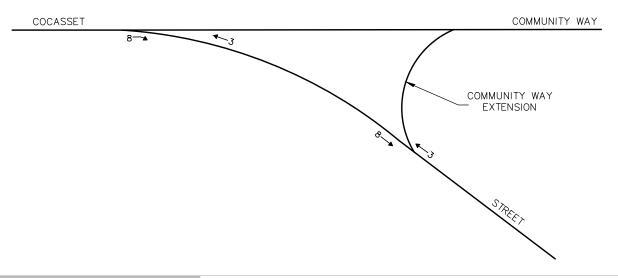
General Background Traffic Growth

														Annual
STA.	CITY/TOWN	ROUTE/STREET	LOCATION	2000	2001	2002	2003	2004	2005	2006	2007	2008	2009	Growth Rate
6671	FOXBOROUGH	RTE, 1	AT WALPOLE T.L.	25100			26700			25900			25100	%80.0-
6733	FOXBOROUGH	RTE 140	NORTH OF FORBES BLVD.			32000			30900			29600		-1.29%
6229	SHARON	NORWOOD ST.	AT WALPOLE T.L.	12600	12600		13300	13600		12000	12100		10700	-1.35%
6699	SHARON	RTE. 27	BTWN. RTE 1 & NORWOOD ST.		7500			8000			8300	2700		-0.45%
6700	SHARON	RTE 27	NORTH OF BAY RD.		17300			17800			13600			-3.85%
														-1.40%



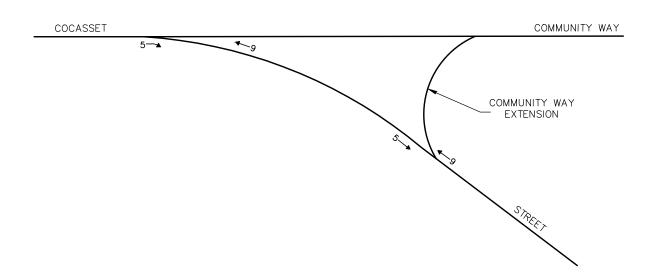
WEEKDAY MORNING PEAK HOUR





WEEKDAY EVENING PEAK HOUR







Background Development River Ridge Residential Peak Hour Traffic Volumes

Figure A-2



Institute of Transportation Engineers (ITE) Trip Generation, 9th Edition Land Use Code (LUC) 210 - Single-Family Detached Housing

Dwelling Units Average Vehicle Trips Ends vs:

Independent Variable (X): 20

AVERAGE WEEKDAY DAILY

```
Ln T = 0.92 Ln (X) + 2.72
Ln T = 0.92 Ln 20 + (2.72)
Ln T = 5.48
  T = 238.91
  T = 240 vehicle trips
    with 50% ( 120 vpd) entering and 50% ( 120 vpd) exiting.
```

WEEKDAY MORNING PEAK HOUR OF ADJACENT STREET TRAFFIC

```
T = 0.70 * (X) + 9.74
T = 0.70 * 20 + (9.74)

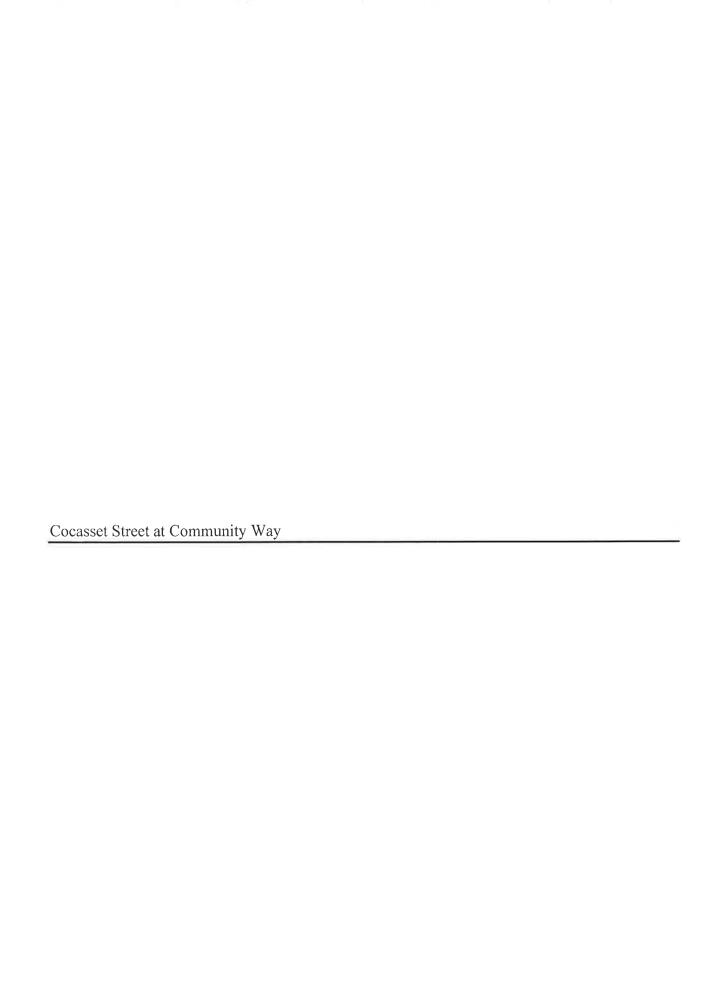
T = 23.74
T = 24 vehicle trips
  with 25% ( 6 vph) entering and 75% ( 18 vph) exiting.
```

WEEKDAY EVENING PEAK HOUR OF ADJACENT STREET TRAFFIC

```
Ln T = 0.90 Ln (X) + 0.51
Ln T = 0.90 Ln 20 + (0.51)
Ln T = 3.21
  T = 24.68
  T = 25 vehicle trips
    with 63% ( 16 vph) entering and 37% ( 9 vph) exiting.
```

CAPACITY ANALYSIS WORKSHEETS

Cocasset Street at Community Way
Cocasset Street at Community Way Extension
Community Way at Community Way Extension and the Project Driveway



	_#	→	4 -	€.	6	4	
Lane Group	EBL	EBT	WBT	WBR	SWL	SWR	
Lane Configurations		4	1		N/A		
Volume (vph)	0	187	437	0	0	3	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	12	12	12	12	11	11	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt					0.865		
Flt Protected							
Satd. Flow (prot)	0	1863	1881	0	1589	0	
Flt Permitted							
Satd. Flow (perm)	0	1863	1881	0	1589	0	
Link Speed (mph)		30	30		30		
Link Distance (ft)		245	216		234		
Travel Time (s)		5.6	4.9		5.3		
Peak Hour Factor	0.84	0.84	0.89	0.89	0.75	0.75	
Heavy Vehicles (%)	0%	2%	1%	0%	0%	0%	
Adj. Flow (vph)	0	223	491	0	0	4	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	223	491	0	4	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Left	Left	Right	Left	Right	
Median Width(ft)		0	0	IMINO MASS	11		
Link Offset(ft)		0	0		0		
Crosswalk Width(ft)		16	16		16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.04	1.04	
Turning Speed (mph)	15			9	15	9	
Sign Control		Free	Free		Stop		
Intersection Summary		II Junesa		II. SVII. SC	YILL YE	a (a) Birin	A RESEARCH
Area Type:	Other						
Control Type: Unsignalized Intersection Capacity Utilizat Analysis Period (min) 15	ion 33.0%			10	CU Level	of Service A	

Intersection			What is	1 P. S.	JE SAYP		#17/WOLVE		VI VIII
Int Delay, s/veh	0.1								
Movement	EBL	EBT	EPO-EV			WBT	WBR	SWL	SWF
Vol, veh/h	0	187				437	0	0	3
Conflicting Peds, #/hr	0	0				0	0	0	(
Sign Control	Free	Free				Free	Free	Stop	Stop
RT Channelized	W W.E.	None				-	None	165	None
Storage Length	2	in the state of th				2		0	
Veh in Median Storage, #	*	0				0	2	0	
Grade, %	-	0				0		0	
Peak Hour Factor	84	84				89	89	75	75
Heavy Vehicles, %	0	2				1	0	0	C
Mvmt Flow	0	223				491	0	0	4
Major/Minor	Major1	- Wisivill				Major2		Minor2	
Conflicting Flow All	491	0	THE PARTY		20,000	Majorz	0	714	491
	491	U					U	491	431
Stage 1	= -	-				-	-	223	
Stage 2	4.4					120		6.4	6.2
Critical Hdwy	4.1	2				-		5.4	0.2
Critical Holy Stg 1	-	7						5.4	
Critical Hdwy Stg 2	0.0					17/2	-	3.5	3.3
Follow-up Hdwy	2.2					198			582
Pot Cap-1 Maneuver	1083					1#3	-	401	362
Stage 1							> ×	619	
Stage 2	**					:=:	-	819	
Platoon blocked, %	4000						•	101	F0(
Mov Cap-1 Maneuver	1083						-	401	582
Mov Cap-2 Maneuver		_3				•		401	
Stage 1						(5)		619	
Stage 2		*						819	
Approach	EB					WB	Yamata V	SW	
HCM Control Delay, s	0					0		11.2	
HCM LOS								В	
Minor Lane/Major Mvmt	EBL	EBT	WBT	WRR	SWLn1	, vg , jv, j	Smill of the last	1/1 25 1000	S. 1 - 1/2 8
Capacity (veh/h)	1083		,,,,,		582				
HCM Lane V/C Ratio	1003	20			0.007				
HCM Control Delay (s)	0	-			11.2				
HCM Lane LOS	A				В.				
					0				
HCM 95th %tile Q(veh)	0	(₩)	*	-	U				

	_#	-	←	~	6	4	
Lane Group	EBL	EBT	WBT	WBR	SWL	SWR	The Market Street Control of Cont
Lane Configurations		4	↑		N/A		
Volume (vph)	4	486	288	0	0	1	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	12	12	12	12	11	11	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt					0.865		
Flt Protected							
Satd. Flow (prot)	0	1881	1881	0	1589	0	
FIt Permitted							
Satd. Flow (perm)	0	1881	1881	0	1589	0	
Link Speed (mph)		30	30		30		
Link Distance (ft)		245	216		234		
Travel Time (s)		5.6	4.9		5.3		
Peak Hour Factor	0.78	0.78	0.93	0.93	0.25	0.25	
Heavy Vehicles (%)	0%	1%	1%	0%	0%	0%	
Adj. Flow (vph)	5	623	310	0	0	4	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	628	310	0	4	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Left	Left	Right	Left	Right	
Median Width(ft)		0	0		11		
Link Offset(ft)		0	0		0		
Crosswalk Width(ft)		16	16		16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.04	1.04	
Turning Speed (mph)	15			9	15	9	
Sign Control		Free	Free		Stop		
Intersection Summary	White S		180,1	CULTAN, 1	8/A3 = 7/A		
Area Type:	Other						
Control Type: Unsignalized Intersection Capacity Utilizat Analysis Period (min) 15	tion 38.8%			10	CU Level	of Service	A

Intersection		A TANK	No ALG	9.65					
Int Delay, s/veh	0.1								
Movement	EBL	EBT				WBT	WBR	SWL	SWF
Vol, veh/h	4	486				288	0	0	
Conflicting Peds, #/hr	0	0				0	0	0	(
Sign Control	Free	Free				Free	Free	Stop	Stop
RT Channelized		None				-	None		None
Storage Length	2	W. T. W. P. D. W.				-	=	0	
Veh in Median Storage, #	9	0				0	2	0	
Grade, %	-	0				0		0	
Peak Hour Factor	78	78				93	93	25	25
Heavy Vehicles, %	0	1				1	0	0	
Mvmt Flow	5	623				310	0	0	4
Major/Minor	Major1	(flynnig)		316	100	Major2		Minor2	No. AND TOTAL
Conflicting Flow All	310	0				(m):	0	943	31
Stage 1	5±7	=				56.5	*	310	
Stage 2		- 1				(4)		633	
Critical Hdwy	4.1					20	2	6.4	6.1
Critical Hdwy Stg 1		-00				-		5.4	
Critical Hdwy Stg 2	(#)					€.	- 3	5.4	
Follow-up Hdwy	2.2	II W. •				:=:		3.5	3.
Pot Cap-1 Maneuver	1262					150		294	73
Stage 1								748	
Stage 2	*	- 2					:=::	533	
Platoon blocked, %		2				196	640.		
Mov Cap-1 Maneuver	1262	-				·	323	292	73
Mov Cap-2 Maneuver						14	25/	292	
Stage 1	:2:					9	-	748	
Stage 2	•					15	050	530	
A	ED.	REW VE	-		175.51	WB		SW	R 70 H. M. C. I. I.
Approach Delevis	EB		III SWI SON		1,500	0		9.9	
HCM Control Delay, s HCM LOS	0.1					U		9.9 A	
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SWLn1		S 3 4 150		
Capacity (veh/h)	1262	-	150	-	735				
HCM Lane V/C Ratio	0.004		-	-	0.005				
HCM Control Delay (s)	7.9	0		-	9.9				
HCM Lane LOS	Α.	A			Α				
HCM 95th %tile Q(veh)	0	-			0				

	_#	-	-	€.	6	4		
Lane Group	EBL	EBT	WBT	WBR	SWL	SWR		
ane Configurations		4Î	1		**			
Volume (vph)	0	208	472	0	0	3		
deal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
ane Width (ft)	12	12	12	12	11	11		
ane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00		
Frt					0.865			
It Protected								
Satd. Flow (prot)	0	1863	1881	0	1589	0		
FIt Permitted								
Satd. Flow (perm)	0	1863	1881	0	1589	0		
ink Speed (mph)		30	30		30			
_ink Distance (ft)		245	216		234			
Travel Time (s)		5.6	4.9		5.3			
Peak Hour Factor	0.84	0.84	0.89	0.89	0.75	0.75		
Heavy Vehicles (%)	0%	2%	1%	0%	0%	0%		
Adj. Flow (vph)	0	248	530	0	0	4		
Shared Lane Traffic (%)	-							
Lane Group Flow (vph)	0	248	530	0	4	0		
Enter Blocked Intersection	No	No	No	No	No	No		
Lane Alignment	Left	Left	Left	Right	Left	Right		
Median Width(ft)	(International	0	0	10000000	11			
Link Offset(ft)		0	0		0			
Crosswalk Width(ft)		16	16		16			
Two way Left Turn Lane								
Headway Factor	1.00	1.00	1.00	1.00	1.04	1.04		
Turning Speed (mph)	15			9	15	9		
Sign Control		Free	Free		Stop			
ntersection Summary		Syruskii.	r Barries		14/9	Mary Carl		
11.77 × 70 × 70 × 11.7	Other							
Control Type: Unsignalized								
Intersection Capacity Utilizat	ion 34.8%			10	CU Level	of Service A		
Analysis Period (min) 15								

Intersection	adme al Cyara			St - 384	28.	1100	EAL A		
Int Delay, s/veh	0.1								
Movement	EBL	EBT				WBT	WBR	SWL	SWF
Vol, veh/h	0	208				472	0	0	
Conflicting Peds, #/hr	0	0				0	0	0	(
Sign Control	Free	Free				Free	Free	Stop	Stor
RT Channelized	W 1900S	None				-	None	=1:001 E = 1	None
Storage Length	2	÷				-	¥	0	
Veh in Median Storage, #	2	0				0	42	0	
Grade, %	-	0				0	20	0	
Peak Hour Factor	84	84				89	89	75	75
Heavy Vehicles, %	0	2				1	0	- 0	(
Mvmt Flow	0	248				530	0	0	
Major/Minor	Major1		i marito		- 183	Major2		Minor2	
Conflicting Flow All	530	0				-	0	778	530
Stage 1	-	_						530	
Stage 2		2				-		248	
Critical Hdwy	4.1	- 2				127	2	6.4	6.:
Critical Hdwy Stg 1						-		5.4	7 L 7 A
Critical Hdwy Stg 2	-					-		5.4	
Follow-up Hdwy	2.2					200		3.5	3.
Pot Cap-1 Maneuver	1048					S#11		368	55
Stage 1	1010						-	594	
Stage 2	:40							798	
Platoon blocked, %						743			
Mov Cap-1 Maneuver	1048	- 4				120	=	368	55
Mov Cap-2 Maneuver						120		368	
Stage 1								594	
Stage 2						:=:	- T 1.	798	
Approach	EB			100 m	, m \$1.50	WB		SW	
HCM Control Delay, s	0					0		11.6	
HCM LOS								В	
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SWLn1	W 1 1 1 1 1			4.6.000
Capacity (veh/h)	1048			-	553				
HCM Lane V/C Ratio	,010			-	0.007				
HCM Control Delay (s)	0				11.6				
HCM Lane LOS	A	1.0			В				
HCM 95th %tile Q(veh)	0	(*)			0				

	#	-	•	€	4	4	
_ane Group	EBL	EBT	WBT	WBR	SWL	SWR	
Lane Configurations		4	^		***		
Volume (vph)	4	526	318	0	0	1	
deal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
ane Width (ft)	12	12	12	12	11	11	
ane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt .					0.865		
It Protected							
Satd. Flow (prot)	0	1881	1881	0	1589	0	
It Permitted							
Satd. Flow (perm)	0	1881	1881	0	1589	0	
ink Speed (mph)		30	30		30		
Link Distance (ft)		245	216		234		
Travel Time (s)		5.6	4.9		5.3		
Peak Hour Factor	0.78	0.78	0.93	0.93	0.25	0.25	
Heavy Vehicles (%)	0%	1%	1%	0%	0%	0%	
Adj. Flow (vph)	5	674	342	0	0	4	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	679	342	0	4	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Left	Left	Right	Left	Right	
Median Width(ft)		0	0		11		
Link Offset(ft)		0	. 0		0		
Crosswalk Width(ft)		16	16		16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.04	1.04	
Turning Speed (mph)	15			9	15	9	
Sign Control		Free	Free		Stop		
ntersection Summary	WALL ST		How S. E.	X FAIR		TIME THE TANK	
Company of the Compan	Other						
Control Type: Unsignalized				16			
Intersection Capacity Utilizat Analysis Period (min) 15	ion 40.9%			10	CU Level	of Service A	

Intersection	STEEL MARKET			T X-18				TELL STORMS TO SE	V. P.X. P.X.
Int Delay, s/veh	0.1								
Movement	EBL	EBT				WBT	WBR	SWL	SWF
Vol, veh/h	4	526				318	0	0	- 1
Conflicting Peds, #/hr	0	0				0	0	0	C
Sign Control	Free	Free				Free	Free	Stop	Stop
RT Channelized	-	None				-	None	3.00	None
Storage Length	50	2				- 2	¥	0	
Veh in Median Storage, #	ě.	0				0	¥	0	
Grade, %		0				0	9	0	
Peak Hour Factor	78	78				93	93	25	25
Heavy Vehicles, %	0	1				1	0	0	(
Mvmt Flow	5	674				342	0	0	4
Major/Minor	Major1		0.00		s depto	Major2	9 2 1 1 1 3 5	Minor2	1019.30
Conflicting Flow All	342	0	No. of Lot, Lot, Lot, Lot, Lot, Lot, Lot, Lot,			Majorz	0	1027	342
	342	U					U	342	042
Stage 1						120	- 19	685	
Stage 2 Critical Hdwy	4.1	-						6.4	6.2
		-						5.4	0.2
Critical Hdwy Stg 1						120		5.4	
Critical Hdwy Stg 2	2.2					(3)		3.5	3.3
Follow-up Hdwy	1228					150		262	705
Pot Cap-1 Maneuver		-						724	700
Stage 1		-				-		504	
Stage 2		141					2	304	
Platoon blocked, %	4000							260	705
Mov Cap-1 Maneuver	1228	S					-	260	70.
Mov Cap-2 Maneuver						٠	3	724	
Stage 1		::::::				67.5	(50)		
Stage 2	•					3	#1	500	
Approach	EB	" - 39° vše	5 ALZ4	Ser' (b	1408	WB		SW	The read
HCM Control Delay, s	0.1					0		10.1	
HCM LOS								В	
Minor Lane/Major Mymt	EBL	EBT	WBT	WBR	SWLn1	12 - U			
Capacity (veh/h)	1228	723	=1	2	705				
HCM Lane V/C Ratio	0.004				0.006				
HCM Control Delay (s)	7.9	0			10.1				
HCM Lane LOS	Α	Ā		-	В				
HCM 95th %tile Q(veh)	0	- A	- 500		0				

	_#	→	←	€.	4	4				
Lane Group	EBL	EBT	WBT	WBR	SWL	SWR		La Marie Mar	TE ST	PENN
Lane Configurations		र्भ	7		N/					
Volume (vph)	4	208	472	0	0	16				
deal Flow (vphpl)	1900	1900	1900	1900	1900	1900				
ane Width (ft)	12	12	12	12	11	11				
ane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00				
-rt					0.865					
FIt Protected		0.999								
Satd. Flow (prot)	0	1862	1881	0	1589	0				
Flt Permitted		0.999								
Satd. Flow (perm)	0	1862	1881	0	1589	0				
ink Speed (mph)		30	30		30					
Link Distance (ft)		245	216		234					
Travel Time (s)		5.6	4.9		5.3					
Peak Hour Factor	0.84	0.84	0.89	0.89	0.75	0.75				
Heavy Vehicles (%)	0%	2%	1%	0%	0%	0%				
Adj. Flow (vph)	5	248	530	0	0	21				
Shared Lane Traffic (%)										
Lane Group Flow (vph)	0	253	530	0	21	0				
Enter Blocked Intersection	No	No	No	No	No	No				
Lane Alignment	Left	Left	Left	Right	Left	Right				
Median Width(ft)		0	0	0.00	11					
Link Offset(ft)		0	0		0					
Crosswalk Width(ft)		16	16		16					
Two way Left Turn Lane										
Headway Factor	1.00	1.00	1.00	1.00	1.04	1.04				
Turning Speed (mph)	15			9	15	9				
Sign Control		Free	Free		Stop					
ntersection Summary	3 10 17						i Sylyidani	377 (11)	Dian.	(ly
71	Other					,				
Control Type: Unsignalized Intersection Capacity Utilizati Analysis Period (min) 15	on 34.8%			10	CU Level	of Service A				

Intersection	and the state of the		1 4.51	0.1	77.	PILST -	Wallet B		
Int Delay, s/veh	0.4								
Movement	EBL	EBT	20V = 110		U5)*3U-	WBT	WBR	SWL	SWF
Vol, veh/h	4	208				472	0	0	1
Conflicting Peds, #/hr	0	0				0	0	0	1.0
Sign Control	Free	Free				Free	Free	Stop	Sto
RT Channelized	-	None				-	None		Non
Storage Length						-		0	
Veh in Median Storage, #	-	0				0		0	
Grade, %	· ·	0				0		0	
Peak Hour Factor	84	84				89	89	75	7
Heavy Vehicles, %	0	2				1	0	0	
Mvmt Flow	5	248				530	0	0	2
Major/Minor	Major1				79 - FW	Major2		Minor2	
Conflicting Flow All	530	0				-	0	787	53
Stage 1						200	-	530	
Stage 2						1,20		257	
Critical Hdwy	4.1					· · · · · · · · · · · · · · · · · · ·		6.4	6.
Critical Hdwy Stg 1	7.1							5.4	
Critical Hdwy Stg 2	71 1						-	5.4	
Follow-up Hdwy	2.2						-21	3.5	3.
Pot Cap-1 Maneuver	1048						141	363	55
Stage 1	1040	20.						594	
Stage 2	-							791	
Platoon blocked, %	: <u></u>	1±.X				:2:		731	
	1048	50						361	55
Mov Cap-1 Maneuver	1040	**				(A.E.)	(-)	361	
Mov Cap-2 Maneuver		-					190	594	
Stage 1	(%)	(a):					3#0	786	
Stage 2	•	•					;#A)	700	
Approach	EB		2903		48.713	WB		SW	
HCM Control Delay, s	0,2					0		11.8	
HCM LOS								В	
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SWLn1				
Capacity (veh/h)	1048		•		553				
HCM Lane V/C Ratio	0.005				0.039				
HCM Control Delay (s)	8.5	0	(#)	~	11.8				
HCM Lane LOS	A	Ä		141	В				
HCM 95th %tile Q(veh)	0	- 20	120		0.1				

	#	→	←	€	(1	
Lane Group	EBL	EBT	WBT	WBR	SWL	SWR	White and the color of the second
Lane Configurations		4	^		**		
Volume (vph)	15	526	318	0	0	7	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	12	12	12	12	11	11	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt					0.865		
FIt Protected		0.999					
Satd. Flow (prot)	0	1880	1881	0	1589	0	
Flt Permitted		0.999					
Satd. Flow (perm)	0	1880	1881	0	1589	0	
Link Speed (mph)		30	30		30		
Link Distance (ft)		245	216		234		
Travel Time (s)		5.6	4.9		5.3		
Peak Hour Factor	0.78	0.78	0.93	0.93	0.25	0.25	
Heavy Vehicles (%)	0%	1%	1%	0%	0%	0%	
Adj. Flow (vph)	19	674	342	0	0	28	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	693	342	0	28	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Left	Left	Right	Left	Right	
Median Width(ft)		0	0		11		
Link Offset(ft)		0	0		0		
Crosswalk Width(ft)		16	16		16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.04	1.04	
Turning Speed (mph)	15			9	15	9	
Sign Control		Free	Free		Stop		
Intersection Summary					, 50 W	20080	Land St. St. St. and St. Hilliam St. 1, 27 Sept.
21	Other						
Control Type: Unsignalized							
Intersection Capacity Utilizat	ion 49.8%	i i		10	CU Level	of Service	A
Analysis Period (min) 15							

Intersection					S STWIN				
Int Delay, s/veh	0.4								
Movement	EBL	EBT	180.0		a 10 10	WBT	WBR	SWL	SWF
Vol, veh/h	15	526				318	0	0	7
Conflicting Peds, #/hr	0	0				0	0	0	(
Sign Control	Free	Free				Free	Free	Stop	Stop
RT Channelized		None				-	None	_	None
Storage Length		-				-		0	
Veh in Median Storage, #	545	0				0		0	
Grade, %	-	0				0		0	
Peak Hour Factor	78	78				93	93	25	25
Heavy Vehicles, %	0	1				1	0	0	(
Mvmt Flow	19	674				342	0	0	28
Major/Minor	Major1	IL SEKIT S		Cont.	or etc. F	Major2	NY STATE	Minor2	
Conflicting Flow All	342	0	rententes.			majora	0	1055	342
Stage 1	042	-					-	342	0.1.
Stage 2								713	
Critical Hdwy	4.1							6.4	6.2
Critical Hdwy Stg 1	74.1	-				-		5.4	0
Critical Hdwy Stg 2	127					125	27	5.4	
Follow-up Hdwy	2.2						-	3.5	3.3
Pot Cap-1 Maneuver	1228	- 2						252	70
Stage 1	1220	3.				171		724	70.
						-	-	489	
Stage 2 Platoon blocked, %		-				<i>7</i> €2	(#)	403	
Mov Cap-1 Maneuver	1220	1.00					-	246	70
	1228						190	246	10.
Mov Cap-2 Maneuver	•	120					*	724	
Stage 1						1.5			
Stage 2		(*)					•	477	
Approach	EB			during.		WB		SW	
HCM Control Delay, s	0.2					0		10.3	
HCM LOS								В	
Minor Lane/Major Mymt	EBL	EBT	WBT	WBR	SWLn1	4 10			Z Synark
Capacity (veh/h)	1228	701	-		705				
HCM Lane V/C Ratio	0.016	Sec. 1			0.04				
HCM Control Delay (s)	8	0	- 47	2	10.3				
HCM Lane LOS	Ä	Ā	-	11	В				
HCM 95th %tile Q(veh)	0	-			0.1				



	*	-	-	*	-	1	
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	
Lane Configurations		4	₽		**		
Volume (vph)	0	187	437	0	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt							
FIt Protected							
Satd. Flow (prot)	0	1863	1863	0	1863	0	
FIt Permitted							
Satd. Flow (perm)	0	1863	1863	0	1863	0	
Link Speed (mph)		30	30		30		
Link Distance (ft)		216	400		82		
Travel Time (s)		4.9	9.1		1.9		
Peak Hour Factor	0.84	0.84	0.89	0.89	0.92	0.92	
Adj. Flow (vph)	0	223	491	0	0	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	223	491	0	0	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Left	Left	Right	Left	Right	
Median Width(ft)		0	0		12		
Link Offset(ft)		0	0		0		
Crosswalk Width(ft)		16	16		16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)	15			9	15	9	
Sign Control		Free	Free		Stop		
Intersection Summary			Hay 3271, 10	1,01,01			
3,1	Other						
Control Type: Unsignalized Intersection Capacity Utilizat	tion 26.3%			10	CU Level	of Service A	

Analysis Period (min) 15

Intersection	W.W.Cau	Fortill's			"Jak V				
Int Delay, s/veh	0								
				esaut M	01015	WEE	WEE	901	ODE
Movement	EBL	EBT				WBT	WBR	SBL	SBR
Vol, veh/h	0	187				437	0	0	C
Conflicting Peds, #/hr	0	0				_ 0	_ 0	0	0
Sign Control	Free	Free				Free	Free	Stop	Stop
RT Channelized	£_	None				-	None	121	None
Storage Length								0	14
Veh in Median Storage, #	<u>=</u>	0				0		0	
Grade, %		0				0	7.	0	1.5
Peak Hour Factor	84	84				89	89	92	92
Heavy Vehicles, %	2	2				2	2	2	2
Mvmt Flow	0	223				491	0	0	(
Major/Minor	Major1				6 ₀ .2.53	Major2	90 W M2.55	Minor2	
Conflicting Flow All	491	0		16. 3			0	714	491
Stage 1	18.501					- 2	¥	491	
Stage 2								223	
Critical Hdwy	4.12					-	-	6.42	6.22
Critical Hdwy Stg 1	11 00 - 1						-	5.42	
Critical Hdwy Stg 2								5.42	
Follow-up Hdwy	2.218						- 1 - 2	3.518	3.318
Pot Cap-1 Maneuver	1072	<u> </u>						398	578
Stage 1		2				- 2		615	
Stage 2		E					2	814	
Platoon blocked, %							2		
Mov Cap-1 Maneuver	1072	=				-		398	578
Mov Cap-2 Maneuver	1072							398	
Stage 1						-		615	
Stage 2	1 - 1 - 1							814	
Approach	EB			= 8.777	WW	WB	w.Ta Will. bi	SB	10° 2000, 2
HCM Control Delay, s	0		- January			0	المتعلقة المراجعة المتعادمة	0	
HCM LOS						U		A	
News W. Williams			27// 800/000	0000VE-verse	234				
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	118 6 8		C STEWNISS SIE	SE PERSON S
Capacity (veh/h)	1072		¥						
HCM Lane V/C Ratio	S P S B			7					
HCM Control Delay (s)	0				0				
HCM Lane LOS	Α	*			Α				
HCM 95th %tile Q(veh)	0	14	-	+:	0.00				

	*	-	←	*	-	1		
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	nt ht. I l	
Lane Configurations		4	₽		***			
Volume (vph)	0	486	288	0	0	0		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00		
Frt								
Flt Protected								
Satd. Flow (prot)	0	1863	1863	0	1863	0		
FIt Permitted								
Satd. Flow (perm)	0	1863	1863	0	1863	0		
Link Speed (mph)		30	30		30			
Link Distance (ft)		216	400		82			
Travel Time (s)		4.9	9.1		1.9			
Peak Hour Factor	0.78	0.78	0.93	0.93	0.92	0.92		
Adj. Flow (vph)	0	623	310	0	0	0		
Shared Lane Traffic (%)								
Lane Group Flow (vph)	0	623	310	0	0	0		
Enter Blocked Intersection	No	No	No	No	No	No		
Lane Alignment	Left	Left	Left	Right	Left	Right		
Median Width(ft)		0	0		12	.50		
Link Offset(ft)		0	0		0			
Crosswalk Width(ft)		16	16		16			
Two way Left Turn Lane								
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00		
Turning Speed (mph)	15			9	15	9		
Sign Control		Free	Free		Stop			
Intersection Summary	si Aliyati	1.0450	13 C. R.	6/35/6	My Pale			

Control Type: Unsignalized

Intersection Capacity Utilization 28.9%

Analysis Period (min) 15

ICU Level of Service A

Intersection			1000	3000	THE REST	a fina			
Int Delay, s/veh	0								
Movement	EBL	EBT				WBT	WBR	SBL	SBF
Vol, veh/h	0	486				288	0	0	(
Conflicting Peds, #/hr	0	0				0	0	0	(
Sign Control	Free	Free				Free	Free	Stop	Stop
RT Channelized	-	None				-	None		None
Storage Length		-						0	
Veh in Median Storage, #		0				0	-	0	
Grade, %	·**	0				0		0	
Peak Hour Factor	78	78				93	93	92	92
Heavy Vehicles, %	2	2				2	2	2	
Mvmt Flow	0	623				310	0	0	(
Major/Minor	Major1	8 89/Mg			T. W. A.	Major2		Minor2	1
Conflicting Flow All	310	0			1 1 1 1 1 1 1 1 1	Wajorz	0	933	310
Stage 1	310	U				7 .	-	310	01,
Stage 2								623	
Critical Hdwy	4.12					: •:		6.42	6.2
Critical Hdwy Stg 1	4.12							5.42	0.2.
Critical Hdwy Stg 2								5.42	
Follow-up Hdwy	2.218							3.518	3.31
Pot Cap-1 Maneuver	1250					787		295	730
Stage 1	1250							744	7.51
Stage 2		2					27	535	
Platoon blocked, %								555	
Mov Cap-1 Maneuver	1250							295	73
	1200					. S.	(B)	295	7.0
Mov Cap-2 Maneuver	*	(m)						744	
Stage 1		:=0					5=8	535	
Stage 2		(4))				-		535	
Approach	EB		Tes /ms=	100	- Aleidi	WB		SB	wa Fish
HCM Control Delay, s HCM LOS	0					0		0 A	
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1				No. 12
Capacity (veh/h)	1250	-	-	-	-				
HCM Lane V/C Ratio	1200	150		- 1					
HCM Control Delay (s)	0	180	-		0				
HCM Lane LOS	A	-	11 1	- û	A				
HCM 95th %tile Q(veh)	ô	120		2	^				

	*	→	•	*	-	1	
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	NEW STATE OF THE S
Lane Configurations		र्स	1		*/*		
Volume (vph)	0	526	318	0	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt							
Flt Protected							
Satd. Flow (prot)	0	1863	1863	0	1863	0	
FIt Permitted							
Satd. Flow (perm)	0	1863	1863	0	1863	0	
Link Speed (mph)		30	30		30		
Link Distance (ft)		216	400		82		
Travel Time (s)		4.9	9.1		1.9		
Peak Hour Factor	0.84	0.84	0.89	0.89	0.92	0.92	
Adj. Flow (vph)	0	626	357	0	0	0	
Shared Lane Traffic (%)	17.0						
Lane Group Flow (vph)	0	626	357	0	0	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Left	Left	Right	Left	Right	
Median Width(ft)	11 (17) (17)	0	0		12	3	
Link Offset(ft)		0	0		0		
Crosswalk Width(ft)		16	16		16		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)	15	- 10.708	27.75	9	15	9	
Sign Control		Free	Free	(8)	Stop	3,	
	WEST DE	17/17/		1 1 8100	e 0.00 0.00	21 6 2 3	THE RESERVE OF THE PARTY OF THE

Intersection Summary

Area Type: Other Control Type: Unsignalized

Intersection Capacity Utilization 31.0%

Analysis Period (min) 15

ICU Level of Service A

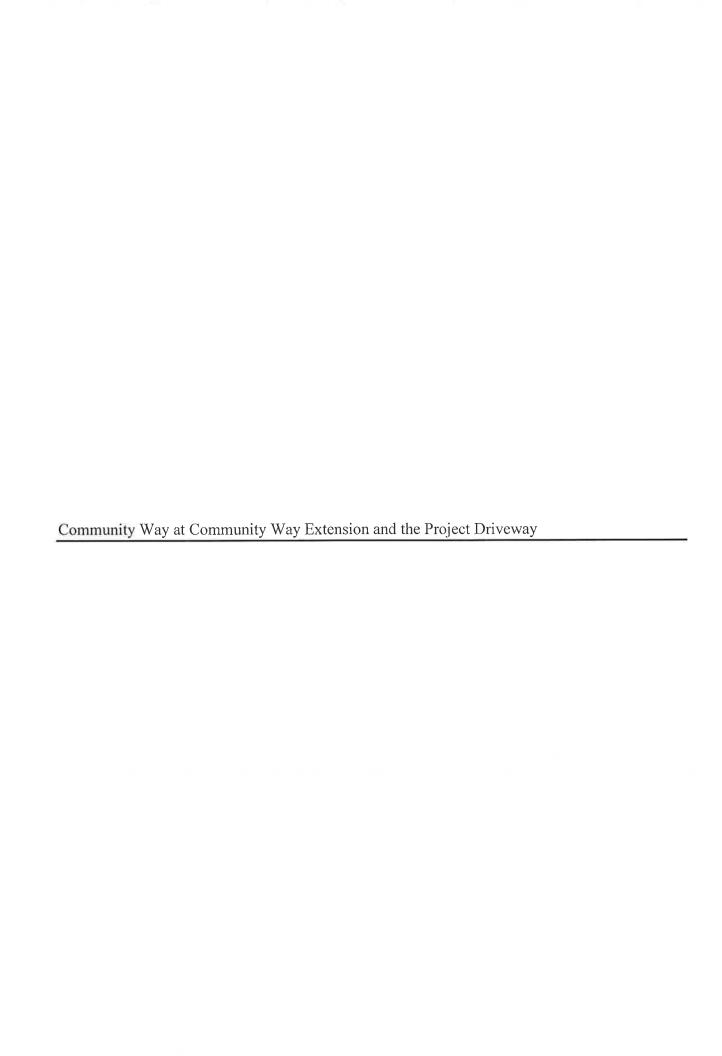
Intersection			02 PK	N. Syll As	1000	- 8 mg	10 2 (8)		
Int Delay, s/veh	0								
Movement	EBL	EBT				WBT	WBR	SBL	SBF
Vol, veh/h	0	526				318	0	0	(
Conflicting Peds, #/hr	0	0				0	0	0	(
Sign Control	Free	Free				Free	Free	Stop	Stop
RT Channelized		None				A MAGA	None	37377#33 8¥3	None
Storage Length	3	_					1.0000100	0	
Veh in Median Storage, #		0				0		0	
Grade, %		0				0		0	
Peak Hour Factor	84	84				89	89	92	92
Heavy Vehicles, %	2	2				2	2	2	
Mvmt Flow	0	626				357	0	0	
Major/Minor	Major1	3-703		-N. 13	-30 III (8. E	Major2	Nacialis a	Minor2	V 20 Tay
Conflicting Flow All	357	0				majorz.	0	983	35
Stage 1	337					-	0	357	00
Stage 2								626	
Critical Hdwy	4.12							6.42	6.2
Critical Hdwy Stg 1	7.12					1		5.42	0.2.
Critical Hdwy Stg 2	_	-						5.42	
Follow-up Hdwy	2.218							3.518	3.31
Pot Cap-1 Maneuver	1202							276	68
Stage 1	1202						2	708	
Stage 2	.21	-				141		533	
Platoon blocked, %		*					3	000	
Mov Cap-1 Maneuver	1202							276	68
Mov Cap-2 Maneuver	1202							276	
Stage 1								708	
Stage 2	100							533	
Approach	EB			094.781.161	i stellere	WB		SB	
HCM Control Delay, s	0		10 11 12			0		0	THE PERMIT
HCM LOS	U					U		A	
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1		1000 5 E		112/08/11
Capacity (veh/h)	1202	-	-	-	-				
HCM Lane V/C Ratio	1202								
HCM Control Delay (s)	0				0				
HCM Lane LOS	A		10 T		A				
HCM 95th %tile Q(veh)	0	-							

	*	-	-	*	-	4						
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	-04 A 6	700				NUN
Lane Configurations		4	1>		M							
Volume (vph)	0	208	472	2	5	0						
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900						
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00						
Frt			0.999									
Flt Protected					0.950							
Satd. Flow (prot)	0	1863	1861	0	1770	0						
FIt Permitted					0.950							
Satd. Flow (perm)	0	1863	1861	0	1770	0						
Link Speed (mph)		30	30		30							
Link Distance (ft)		216	400		82							
Travel Time (s)		4.9	9.1		1.9							
Peak Hour Factor	0.84	0.84	0.89	0.89	0.92	0.92						
Adj. Flow (vph)	0	248	530	2	5	0						
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	248	532	0	5	0						
Enter Blocked Intersection	No	No	No	No	No	No						
Lane Alignment	Left	Left	Left	Right	Left	Right						
Median Width(ft)		0	0		12							
Link Offset(ft)		0	0		0							
Crosswalk Width(ft)		16	16		16							
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00						
Turning Speed (mph)	15			9	15	9						
Sign Control		Free	Free		Stop							
Intersection Summary	10/20			98.5	alt (wall	THE PERSON	3/6/8 (9)	XTIME S	S 14 A S	12 B	J. W.	V TOY
A CONTRACTOR OF THE PARTY OF TH	Other											
Control Type: Unsignalized Intersection Capacity Utilizat Analysis Period (min) 15	tion 35.0%			K	CU Level	of Service A						

Intersection		Z# 7/8/10			7 - 18 _ N			A TOTAL	
Int Delay, s/veh	0.1								
Movement	EBL	EBT				WBT	WBR	SBL	SBF
Vol, veh/h	0	208				472	2	5	(
Conflicting Peds, #/hr	0	0				0	0	0	(
Sign Control	Free	Free				Free	Free	Stop	Stop
RT Channelized	-	None					None		None
Storage Length	=					5 7 0	-	0	
Veh in Median Storage, #		0				0	-	0	
Grade, %	-	0				0		0	
Peak Hour Factor	84	84				89	89	92	92
Heavy Vehicles, %	2	2				2	2	2	
Mvmt Flow	0	248				530	2	5	(
Major/Minor	Major1	3 3 3		DE WE		Major2	eve il visiva	Minor2	Same of the
Conflicting Flow All	533	0				-	0	779	531
Stage 1	- 000	-				- :		531	00
Stage 2						190		248	
Critical Hdwy	4.12					3=1		6.42	6.22
Critical Hdwy Stg 1								5.42	0.0.
Critical Hdwy Stg 2								5.42	
Follow-up Hdwy	2.218							3.518	3.318
Pot Cap-1 Maneuver	1035	4				:=:	-	364	548
Stage 1	1000					12		590	
Stage 2							•	793	
Platoon blocked, %						11.			
Mov Cap-1 Maneuver	1035							364	548
Mov Cap-2 Maneuver	1000	-				WITE.		364	18 V
Stage 1	-						:=0	590	
Stage 2		-				1 200	(#KL)	793	
Approach	EB	TA SELECTION	- N. 5 % 2	L -300	4.5.4.5	WB		SB	- A TOTAL ***
HCM Control Delay, s	0					0		15	
HCM LOS								C	
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1				
Capacity (veh/h)	1035	:33		-	364				
HCM Lane V/C Ratio	ewith .				0.015				
HCM Control Delay (s)	0	(#)		-	15				
HCM Lane LOS	A	(4)	-	- " -	С				
HCM 95th %tile Q(veh)	0	120	- 2	¥	0				

	۶	→	←	4	>	1			
Lane Group	EBL	EBT	WBT	WBR	SBL	SBR	MAN (12)		
Lane Configurations		4	1→		**				
Volume (vph)	0	526	318	5	3	0			
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900			
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00			
Frt			0.998						
Flt Protected					0.950				
Satd. Flow (prot)	0	1863	1859	0	1770	0			
FIt Permitted					0.950				
Satd. Flow (perm)	0	1863	1859	0	1770	0			
Link Speed (mph)		30	30		30				
Link Distance (ft)		216	400		82				
Travel Time (s)		4.9	9.1		1.9				
Peak Hour Factor	0.78	0.78	0.93	0.93	0.92	0.92			
Adj. Flow (vph)	0	674	342	5	3	0			
Shared Lane Traffic (%)									
Lane Group Flow (vph)	0	674	347	0	3	0			
Enter Blocked Intersection	No	No	No	No	No	No			
Lane Alignment	Left	Left	Left	Right	Left	Right			
Median Width(ft)		0	0		12				
Link Offset(ft)		0	0		0				
Crosswalk Width(ft)		16	16		16				
Two way Left Turn Lane									
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00			
Turning Speed (mph)	15			9	15	9			
Sign Control		Free	Free		Stop				
Intersection Summary	THE STATE OF THE				588 JF 1		A	with the	
21	Other								
Control Type: Unsignalized									
Intersection Capacity Utilizat	ion 37.7%			10	CU Level	of Service A			
Analysis Period (min) 15									

Intersection			ST WILL	San C		- Madin			A Payor
Int Delay, s/veh	0.1								
Movement	EBL	EBT		3,000	ymus.	WBT	WBR	SBL	SBF
Vol, veh/h	0	526	-			318	5	3	(
Conflicting Peds, #/hr	0	0				0	0	0	(
Sign Control	Free	Free				Free	Free	Stop	Stop
RT Channelized		None				-	None	-	None
Storage Length							#	0	
Veh in Median Storage, #		0				0	*	0	
Grade, %	12	0				0	-	0	
Peak Hour Factor	78	78				93	93	92	92
Heavy Vehicles, %	2	2				2	2	2	
Mvmt Flow	ō	674				342	5	3	(
Major/Minor	Major1		asie il Lin	mienuono		Major2		Minor2	
Major/Minor		0	- 10			Majurz	0		34
Conflicting Flow All	347	0					0	1019	343
Stage 1	•	-						345	
Stage 2	4.40							674	0.00
Critical Hdwy	4.12	¥				(*)		6.42	6.22
Critical Hdwy Stg 1	*					*	2	5.42	
Critical Hdwy Stg 2		¥				•	-	5.42	0.04
Follow-up Hdwy	2.218	1						3.518	3.318
Pot Cap-1 Maneuver	1212							263	698
Stage 1	- ::						:7	717	
Stage 2	(♥).					: * ::	:=	506	
Platoon blocked, %						13.0			100.00
Mov Cap-1 Maneuver	1212	*				140	-	263	698
Mov Cap-2 Maneuver		-				240	*	263	
Stage 1	:20	-				:#3	-2	717	
Stage 2	9					*		506	
Approach	EB					WB		SB	SYTHOUGH SE
HCM Control Delay, s	0					0		18.9	
HCM LOS								С	
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	30.705		1274 LE 011 LE	3 82 FL 511
Capacity (veh/h)	1212	:•:	-	*:	263				
HCM Lane V/C Ratio		100		ų.	0.012				
HCM Control Delay (s)	0		2	2	18.9				
HCM Lane LOS	Ä	20		-	C				
HCM 95th %tile Q(veh)	Ô	•			0				



	-	•	•	-	4	/	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	ĵ.			र्स	*A		
Volume (vph)	0	0	0	3	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	11	11	11	11	12	12	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt							
Flt Protected							
Satd. Flow (prot)	1801	0	0	1801	1863	0	
Flt Permitted							
Satd. Flow (perm)	1801	0	0	1801	1863	0	
Link Speed (mph)	30			30	30		
Link Distance (ft)	234			482	82		
Travel Time (s)	5.3			11.0	1.9		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	0	0	0	3	0	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	0	0	3	0	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Left	Left	Right	
Median Width(ft)	0			0	12		
Link Offset(ft)	0			0	0		
Crosswalk Width(ft)	16			16	16		
Two way Left Turn Lane							
Headway Factor	1.04	1.04	1.04	1.04	1.00	1.00	
Turning Speed (mph)		9	15		15	9	
Sign Control	Free			Free	Stop		
Intersection Summary			How !!	Cress (4)		See That's	
	Other ion 6.7%			IC	CU Level	of Service A	

Intersection								
Int Delay, s/veh	0							
Movement		EBT	EBR	WB	Ĺ	WBT	NBL	NBF
Vol, veh/h		0	0		0	3	0	(
Conflicting Peds, #/hr		0	0	10	0	0	0	(
Sign Control		Free	Free	Fre		Free	Stop	Stop
RT Channelized			None			None		None
Storage Length						3	0	
Veh in Median Storage, #		0	1.5			0	0	
Grade, %		0				0	0	
Peak Hour Factor		92	92	9	2	92	92	92
Heavy Vehicles, %		2	2		2	2	2	
Mvmt Flow		0	0		0	3	0	(
Major/Minor		Major1		Major	2		Minor1	
Conflicting Flow All		0	0		0	0	3	(
Stage 1						9	0	
Stage 2		v * 15				- 3	3	
Critical Hdwy				4.1	2		6.42	6.22
Critical Hdwy Stg 1						1 / 1	5.42	
Critical Hdwy Stg 2		*	:(•:				5.42	
Follow-up Hdwy			16	2.21	8		3.518	3.318
Pot Cap-1 Maneuver		2	14		-	¥	1019	
Stage 1		- 2	1				4 - 2 - 1 - 1 - 1	
Stage 2						-	1020	
Platoon blocked, %		V 3	- 174					
Mov Cap-1 Maneuver							1019	
Mov Cap-2 Maneuver		7			()		1019	
Stage 1								
Stage 2		¥					1020	
Approach		EB	1 J. 16	W	В		NB	n just
HCM Control Delay, s		0			0		0	
HCM LOS							A	
Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL WB	T			31 10 2 3
Capacity (veh/h)	•			-	œ.			
HCM Lane V/C Ratio		0 1						
HCM Control Delay (s)	0	-		0	3 * 2			
HCM Lane LOS	Ä	-	*	A	000			
HCM 95th %tile Q(veh)	.63% -	-		22				

	-	*	1	←	4	1	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	\$			4	**/*		
Volume (vph)	4	0	0	1	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	11	11	11	11	12	12	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt							
Flt Protected							
Satd. Flow (prot)	1801	0	0	1801	1863	0	
FIt Permitted							
Satd. Flow (perm)	1801	0	0	1801	1863	0	
Link Speed (mph)	30			30	30		
Link Distance (ft)	234			482	82		
Travel Time (s)	5.3			11.0	1.9		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	4	0	0	1	0	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	4	0	0	1	0	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Left	Left	Right	
Median Width(ft)	0			0	12		
Link Offset(ft)	0			0	0		
Crosswalk Width(ft)	16			16	16		
Two way Left Turn Lane							
Headway Factor	1.04	1.04	1.04	1.04	1.00	1.00	
Turning Speed (mph)		9	15		15	9	
Sign Control	Free			Free	Stop		
Intersection Summary							
Area Type: C Control Type: Unsignalized Intersection Capacity Utilizati	on 6.7%			10	'III aval	of Service	A

Intersection	PRINTED BY		A DOME			- V - 1816		E SWITTS
Int Delay, s/veh	0							
Movement		EBT	EBR		WBL	WBT	NBL	NBF
Vol, veh/h		4	0		0	1	0	
Conflicting Peds, #/hr		0	0		0	0	0	(
Sign Control		Free	Free		Free	Free	Stop	Stop
RT Channelized			None		123	None		None
Storage Length					-		0	
Veh in Median Storage, #		0			-	0	0	
Grade, %		0	+			0	0	
Peak Hour Factor		92	92		92	92	92	92
Heavy Vehicles, %		2	2		2	2	2	2
Mvmt Flow		4	0		0	1	0	(
Major/Minor	THE SECOND S	Major1	8-871		Major2	A Interior	Minor1	
Conflicting Flow All		0	0		4	0	5	
Stage 1			-		.580		4	
Stage 2		- 5 - 1 U					1	
Critical Hdwy					4.12	190	6.42	6.22
Critical Hdwy Stg 1					7.12		5.42	0.22
Critical Hdwy Stg 2			2				5.42	
Follow-up Hdwy					2.218		3.518	3.318
Pot Cap-1 Maneuver			-		1618		1017	1080
			- :				1017	1000
Stage 1			IL SI			•	1022	
Stage 2		Æ			0.55	(5)	1022	
Platoon blocked, %			11 27 %		4040	250	1017	4000
Mov Cap-1 Maneuver					1618	(₩)	1017	1080
Mov Cap-2 Maneuver		-				-	1017	
Stage 1			¥		10 4 5		1019	
Stage 2		2					1022	
Approach		EB		500	WB	on the season	NB	Silvern coline
HCM Control Delay, s		0			0		0	
HCM LOS							Α	
Minor Lane/Major Mymt	NBLn1	EBT	EBR	WBL	WBT			
Capacity (veh/h)		-	*	1618	-			
HCM Lane V/C Ratio	:•:		-	-	1 4			
HCM Control Delay (s)	0			0				
HCM Lane LOS	Ä	- 2	5 2	A				
HCM 95th %tile Q(veh)		= 7.		0				

Lane Group					- 1	- 1"	
	EBT	EBR	WBL	WBT	NBL	NBR	Carlos Vices de La Sanda de Carlos
Lane Configurations	₽			4	*/*		
Volume (vph)	0	0	0	3	0	0	
	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	11	11	11	11	12	12	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt							
Flt Protected							
Satd. Flow (prot)	1801	0	0	1801	1863	0	
FIt Permitted							
Satd. Flow (perm)	1801	0	0	1801	1863	0	
Link Speed (mph)	30			30	30		
Link Distance (ft)	234			482	82		
Travel Time (s)	5.3			11.0	1.9		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	0	0	0	3	0	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	0	0	0	3	0	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Left	Left	Right	
Median Width(ft)	0			0	12		
Link Offset(ft)	0			0	0		
Crosswalk Width(ft)	16			16	16		
Two way Left Turn Lane							
Headway Factor	1.04	1.04	1.04	1.04	1.00	1.00	
Turning Speed (mph)		9	15		15	9	
Sign Control	Free			Free	Stop		
	(CIPIE)	WALL DE				, 4 Y.W. E.,	

Intersection			Deg/Es					
Int Delay, s/veh	0							
Movement	SIESURI 76	EBT	EBR		WBL	WBT	NBL	NBF
Vol, veh/h		0	0		0	3	0	(
Conflicting Peds, #/hr		0	0		0	0	0	(
Sign Control		Free	Free		Free	Free	Stop	Stop
RT Channelized		-	None			None		None
Storage Length		-			-	-	0	
Veh in Median Storage, #		0	8.00			0	0	
Grade, %		0	196			0	0	
Peak Hour Factor		92	92		92	92	92	92
Heavy Vehicles, %		2	2		2	2	2	2
Mvmt Flow		0	0		0	3	0	(
Major/Minor		Major1	LYSLE.		Major2	1 /40	Minor1	
Conflicting Flow All		0	0		0	0	3	(
Stage 1					-	-	0	
Stage 2			1.50			- 1	3.	
Critical Hdwy					4.12		6.42	6.22
Critical Hdwy Stg 1					7.12		5.42	0.2
Critical Hdwy Stg 2					740		5.42	
Follow-up Hdwy		2			2.218		3.518	3.318
Pot Cap-1 Maneuver					2.210	a	1019	0.010
Stage 1					1		1013	
Stage 2		:					1020	
Platoon blocked, %					1,753		1020	
Mov Cap-1 Maneuver			2				1019	
Mov Cap-1 Maneuver					100		1019	
Stage 1							1019	
Stage 2					1.51		1020	
Stage 2			-			(4)	1020	
Approach		EB			WB	r sac	NB	TH 115 FINE D
HCM Control Delay, s		0			0		0	
HCM LOS							A	
Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT			TIPE (97 (24)
Capacity (veh/h)	-		-	10:	-			
HCM Lane V/C Ratio		9			1 30			
HCM Control Delay (s)	0	-	-	0	3#(
HCM Lane LOS	Ä			Ā				
HCM 95th %tile Q(veh)		- 2		-	540			

	→	*	*	•	4	<i>></i>	
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	f)			र्स	**		
Volume (vph)	4	0	0	1	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Lane Width (ft)	11	11	11	11	12	12	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Frt							
FIt Protected							
Satd. Flow (prot)	1801	0	0	1801	1863	0	
FIt Permitted							
Satd. Flow (perm)	1801	0	0	1801	1863	0	
Link Speed (mph)	30			30	30		
Link Distance (ft)	234			482	82		
Travel Time (s)	5.3			11.0	1.9		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	4	0	0	1	0	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	4	0	0	1	0	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Right	Left	Left	Left	Right	
Median Width(ft)	0	-		0	12		
Link Offset(ft)	0			0	0		
Crosswalk Width(ft)	16			16	16		
Two way Left Turn Lane							
Headway Factor	1.04	1.04	1.04	1.04	1.00	1.00	
Turning Speed (mph)		9	15		15	9	
Sign Control	Free			Free	Stop		
Intersection Summary				ra di ina		70 70 7 7 7	
Area Type: Control Type: Unsignalized Intersection Capacity Utilizat Analysis Period (min) 15	Other ion 6.7%			IC	CU Level	of Service A	

Intersection	1084 1941 1	200	N S SYOT	1757	1000	18111911884		
Int Delay, s/veh	0							
Movement		EBT	EBR		WBL	WBT	NBL	NBF
Vol, veh/h		4	0		0	1	0	C
Conflicting Peds, #/hr		0	0		0	0	0	C
Sign Control		Free	Free		Free	Free	Stop	Stop
RT Channelized		T	None			None		None
Storage Length		E					0.	
Veh in Median Storage, #		0			(25)	0	0	
Grade, %		0				0	0	
Peak Hour Factor		92	92		92	92	92	92
Heavy Vehicles, %		2	2		2	2	2	2
Mvmt Flow		4	0		0	1	0	C
Major/Minor		Major1	MI V SI	N	//ajor2		Minor1	
Conflicting Flow All		0	0	*	4	0	5	4
Stage 1		-	-		10 C	-	4	
Stage 2		5111 J			S=1	:=:	1	
Critical Hdwy		50			4.12	:•:	6.42	6.22
Critical Hdwy Stg 1					0+0	(*)	5.42	
Critical Hdwy Stg 2		_			(#)	-	5.42	
Follow-up Hdwy		12	4		2.218	1 (8)	3.518	3.318
Pot Cap-1 Maneuver		2	2		1618	120	1017	1080
Stage 1		10 21	- 12		1010		1019	
Stage 2			-				1022	
Platoon blocked, %						8.0		
Mov Cap-1 Maneuver					1618		1017	1080
Mov Cap-2 Maneuver					-		1017	
Stage 1		-					1019	
Stage 2		- 18	2"		4		1022	
Approach	Wales of R. Con	EB			WB	S = - 11 11 1 2 x	NB	
HCM Control Delay, s		0			0		0	
HCM LOS							A	
Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT			TO BUT WELL
Capacity (veh/h)	est Market Part			1618	5.			
HCM Lane V/C Ratio	1							
HCM Control Delay (s)	0		*	0				
HCM Lane LOS	Ă			A				
HCM 95th %tile Q(veh)	200	140	4	Ö	4			

	*	→	•	•	4-	*	4	†	<i>></i>	-	1	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			44>			4	
Volume (vph)	4	1	0	0	3	0	0	2	0	0	5	13
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	11	11	11	12	12	12	12	12	12
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt											0.901	
Flt Protected		0.962										
Satd. Flow (prot)	0	1732	0	0	1801	0	0	1863	0	0	1678	0
FIt Permitted		0.962										
Satd. Flow (perm)	0	1732	0	0	1801	0	0	1863	0	0	1678	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		234			482			82			197	
Travel Time (s)		5.3			11.0			1.9			4.5	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	4	1	0	0	3	0	0	2	0	0	5	14
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	5	0	0	3	0	0	2	0	0	19	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0	1117		0	100		0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.04	1.04	1.04	1.04	1.04	1.04	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Free			Free			Stop			Stop	
Intersection Summary						Dell'a All	L CY LET	ST TOP	#3 /M		i isimi	H itad
	Other	e hin				1	- 1					
Control Type: Unsignalized					CU WILLIAM COMMENT	of Comico						

Intersection Capacity Utilization 13.5%

ICU Level of Service A

Analysis Period (min) 15

Int Delay, s/veh	7.2										
Movement	EBL	EBT	EBR	U V S.E.R.	WBL	WBT	WBR	etwi ego	NBL	NBT	NBF
Vol, veh/h	4	1	0		0	3	0		0	2	(
Conflicting Peds, #/hr	0	0	0		0	0	0		0	0	(
Sign Control	Free	Free	Free		Free	Free	Free		Stop	Stop	Stop
RT Channelized	-	-	None			-	None			-	None
Storage Length	(9)				-					-	
Veh in Median Storage, #	**	0			858	0				0	
Grade, %		0			:*:	0				0	
Peak Hour Factor	92	92	92		92	92	92		92	92	92
Heavy Vehicles, %	2	2	2		2	2	2		2	2	2
Mvmt Flow	4	1	0		0	3	0		0	2	(
Major/Minor	Major1	PISSON.	- 2.2.4	Wall on	Najor2		1.0 E	7.5%.7	Minor1	900	
Conflicting Flow All	3	0	0		1	0	0		23	13	
Stage 1	-		-		-	-			10	10	
Stage 2									13	3	
Critical Hdwy	4.12	*			4.12				7.12	6.52	6.22
Critical Hdwy Stg 1	11.	W				740	- 7		6.12	5.52	
Critical Hdwy Stg 2	(4)				(æ)	(#)	_		6.12	5.52	
Follow-up Hdwy	2.218	20	u u		2.218	-			3.518	4.018	3.318
Pot Cap-1 Maneuver	1619	2	- 5		1622	540	ų.		989	881	1084
Stage 1		-				(6)			1011	887	
Stage 2					-				1007	893	
Platoon blocked, %									1,0,0,1		
Mov Cap-1 Maneuver	1619		- 121		1622				970	879	1084
Mov Cap-2 Maneuver	1010				-				970	879	100
Stage 1		-	_		0140		_		1009	885	
Stage 2		11 20	-		8		-		988	893	
Approach	EB	W- B		TANK ATA	WB	, general		F S V. 15.	NB	158 45	866
HCM Control Delay, s	5.8				0	75			9.1		
HCM LOS	3.0				U				A		
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)	879	1619	-	-	1622		:=:	1016			
HCM Lane V/C Ratio	0.002	0.003			-		997	0.019			
HCM Control Delay (s)	9.1	7.2	0		0	:•1	:=0	8.6			
HCM Lane LOS	Α	Α.Δ	A		A	5#1	(4.)	Α.			
HCM 95th %tile Q(veh)	Ô	0		-	Ô	:=:	120	0.1			

Intersection Int Delay, s/veh				
in Doidy, Siven				
Movement	SBL	SBT	SBR	
Vol, veh/h	0	5	13	
Conflicting Peds, #/hr	0	0	0	
Sign Control	Stop	Stop	Stop	
RT Channelized	1	ш	None	
Storage Length				
Veh in Median Storage, #	5	0	1/5	
Grade, %		0		
Peak Hour Factor	92	92	92	
Heavy Vehicles, %	2	2	2	
Mvmt Flow	0	5	14	
Major/Minor	Minor2	16.57		
Conflicting Flow All	14	13	3	
Stage 1	3	3	- 5	
Stage 2	11	10		
Critical Hdwy	7.12	6.52	6.22	
Critical Hdwy Stg 1	6.12	5.52		
Critical Hdwy Stg 2	6.12	5.52		
Follow-up Hdwy	3.518	4.018	3.318	
Pot Cap-1 Maneuver	1002	881	1081	
Stage 1	1020	893	-	
Stage 2	1010	887	2	
Platoon blocked, %				
Mov Cap-1 Maneuver	999	879	1081	
Mov Cap-2 Maneuver	999	879		
Stage 1	1018	893		
Stage 2	1006	885		
Approach	SB	A PERMI		
HCM Control Delay, s	8.6			
HCM LOS	A			

2: Community Way Ext/Project Driveway & Community Way

	*	→	*	•	←	*	4	†	1	-	↓	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		44			4			43+			44	
Volume (vph)	11	4	0	0	1	0	0	5	0	0	3	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	11	11	11	12	12	12	12	12	12
Lane Util, Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt											0.905	
FIt Protected		0.964										
Satd. Flow (prot)	0	1736	0	0	1801	0	0	1863	0	0	1686	0
Flt Permitted		0.964										
Satd. Flow (perm)	0	1736	0	0	1801	0	0	1863	0	0	1686	0
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		234			482			82			159	
Travel Time (s)		5.3			11.0			1.9			3.6	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	12	4	0	0	1	0	0	5	0	0	3	7
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	16	0	0	1	0	0	5	0	0	10	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0	10.75		0			0	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.04	1.04	1.04	1.04	1.04	1.04	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Sign Control		Free			Free			Stop			Stop	
Intersection Summary				200	li plica	ikilija).	Why.	×	William St.	windy of	Couples	
Area Type: (Other											

Control Type: Unsignalized

Intersection Capacity Utilization 17.5%

ICU Level of Service A

Analysis Period (min) 15

Int Delay, s/veh	6.8										
Movement	EBL	EBT	EBR	res inco	WBL	WBT	WBR	2100202	NBL	NBT	NBF
Vol, veh/h	11	4	0	1 J// 2 2 1 1	0	1	0		0	5	(VDI
Conflicting Peds, #/hr	0	0	0		0	Ö	0		0	0	
Sign Control	Free	Free	Free		Free	Free	Free		Stop	Stop	Sto
RT Channelized	1166	1166	None		1166	1166	None		Оюр	Olop	None
Storage Length		-	None				NOHE			- 5	INOIT
Veh in Median Storage, #		0	7/20		:=:	0			151	0	
Grade, %		0	100		-	0	7		-	0	
Peak Hour Factor	92	92	92		92	92	92		92	92	9
	2	2	2		2	2	2		2	2	3.
Heavy Vehicles, %	12	4				1	0		0	5	(
Mvmt Flow	12	4	0		0		U		U	5	,
Major/Minor	Major1			N	lajor2		Artill S	I Assess	Minor1		Milit
Conflicting Flow All	1	0	0		4	0	0		34	29	
Stage 1		-	-		÷	-			28	28	
Stage 2		-							6	1	
Critical Hdwy	4.12				4.12		-		7.12	6.52	6.2
Critical Hdwy Stg 1					-				6.12	5.52	
Critical Hdwy Stg 2					-		-		6.12	5.52	
Follow-up Hdwy	2.218	=			2.218	941			3.518	4.018	3.31
Pot Cap-1 Maneuver	1622	- 2	2		1618	iani			973	864	108
Stage 1	10 20					120	- 2		989	872	
Stage 2	-								1016	895	
Platoon blocked, %									Kelley.		
Mov Cap-1 Maneuver	1622		-		1618		-		959	858	108
Mov Cap-2 Maneuver	1022				1010				959	858	100
Stage 1	180	-				(*)	-		982	866	
Stage 2			2		721	120			1006	895	
Stage 2		-				-			1000	000	
Approach	EB				WB		ins vic	1118 July 11	NB		1-808
HCM Control Delay, s	5.3				0				9.2		
HCM LOS									Α		
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1	Parts.	THE STATE	
Capacity (veh/h)	858	1622	-		1618		-	997			
HCM Lane V/C Ratio	0.006	0.007			1010	-	-	0.01			
HCM Control Delay (s)	9.2	7.2	0		0			8.6			
HCM Lane LOS	3.2 A	Α.Α	A		A			Α.			
HCM 95th %tile Q(veh)	0	0		2	0		-	0			

Int Delay, s/veh										
Movement	SBL	SBT	SBR	9000		W Vi		V E		
Vol, veh/h	0	3	6							
Conflicting Peds, #/hr	0	0	0							
Sign Control	Stop	Stop	Stop							
RT Channelized	, -	-	None							
Storage Length	-	-	-							
Veh in Median Storage, #	_	0	-							
Grade, %	-	0	-							
Peak Hour Factor	92	92	92							
Heavy Vehicles, %	2	2	2							
Mvmt Flow	0	3	7							
Major/Minor	Minor2					18-201	4 fi Su	THE RE	186	
Conflicting Flow All	32	29	1							
Stage 1	1	1	-							
Stage 2	31	28	.v. š							
Critical Hdwy	7.12	6.52	6.22							
Critical Hdwy Stg 1	6.12	5.52								
Critical Hdwy Stg 2	6.12	5.52	-							
Follow-up Hdwy	3.518	4.018	3.318							
Pot Cap-1 Maneuver	976	864	1084							
Stage 1	1022	895	4							
Stage 2	986	872	22							
Platoon blocked, %										
Mov Cap-1 Maneuver	966	858	1084							
Mov Cap-2 Maneuver	966	858								
Stage 1	1015	895								
Stage 2	973	866								
A					110000000000000000000000000000000000000	- 14 H			100000000000000000000000000000000000000	1000
Approach	SB	SHIP IT			316 87		AND THE ST	ETEROS NE I		
HCM Control Delay, s	8.6									
HCM LOS	Α									